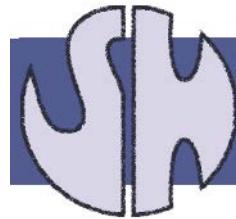




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STEP LENGTH INFLUENCE IN MODELLING ADVECTION AND DIFFUSION OF BED-LOAD PARTICLES

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- Introduction
- Model
- Simulation
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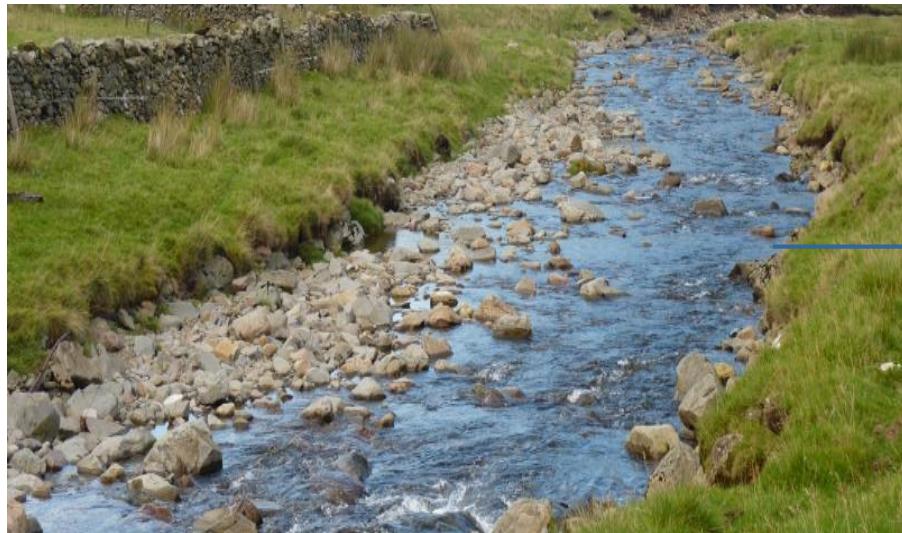
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= Hydrodynamic Transport in Ecologically Critical Heterogeneous Interfaces

Rivers bed = water-sediment interface

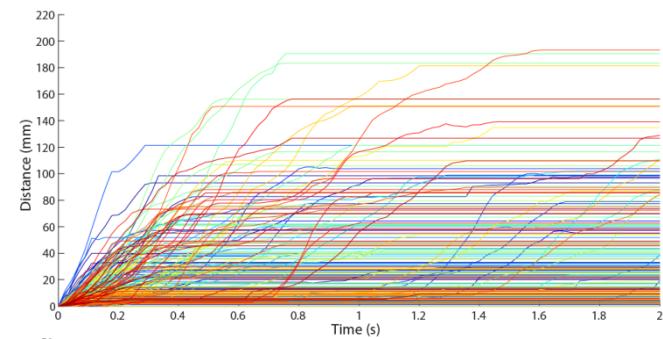
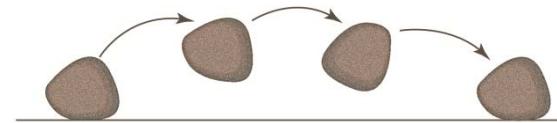


- Exchanging processes
- Potential contamination

HOW SEDIMENT MOVES?



- Field & lab experiments with bed load tracers
- Heterogeneity along travelling path
- From a deterministic approach → to a more appropriate stochastic analysis of motion
- Diffusion emerges due to turbulence and bed roughness [1,2,3]



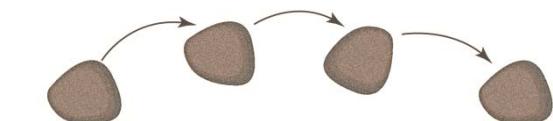
- [1] Nikora et al., Water Resour. Res. 2002;
[2] Ganti et al., J. Geophys. Res. 2010;
[3] Furbish et al., J. Geophys. Res. 2012.



OBJECTIVE:

investigate characteristics of bedload sediment motion to model advective and diffusive transport behaviour

By studying particles' step length = travel distance from pick up to deposition





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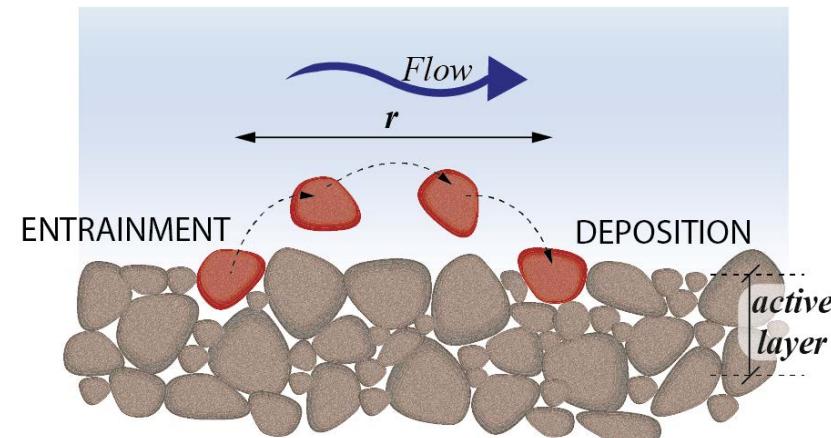
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Exner's equation in entrainment E and deposition D terms

$$(1 - \lambda_p) \frac{\partial \eta(x, t)}{\partial t} = D(x, t) - E(x, t)$$

η = local bed elevation
 λ_p = bed porosity



$$D(x, t) = \int_0^{\infty} E\left(x - r, t - \frac{r}{v_p}\right) f_s(r) dr$$

Stochastically represented

r/v_p = particle travelling time



- Using tracers as representative of entire population

f_T = fraction of tracers

$$\begin{cases} E_T(x, t) = E(x, t)f_T(x, t) \\ D_T(x, t) = \int_0^{\infty} E\left(x - r, t - \frac{r}{v_p}\right) f_T\left(x - r, t - \frac{r}{v_p}\right) f_S(r) dr \end{cases}$$

- Bed in equilibrium: only active layer L_a exchanges sediment with bed load transport [4]

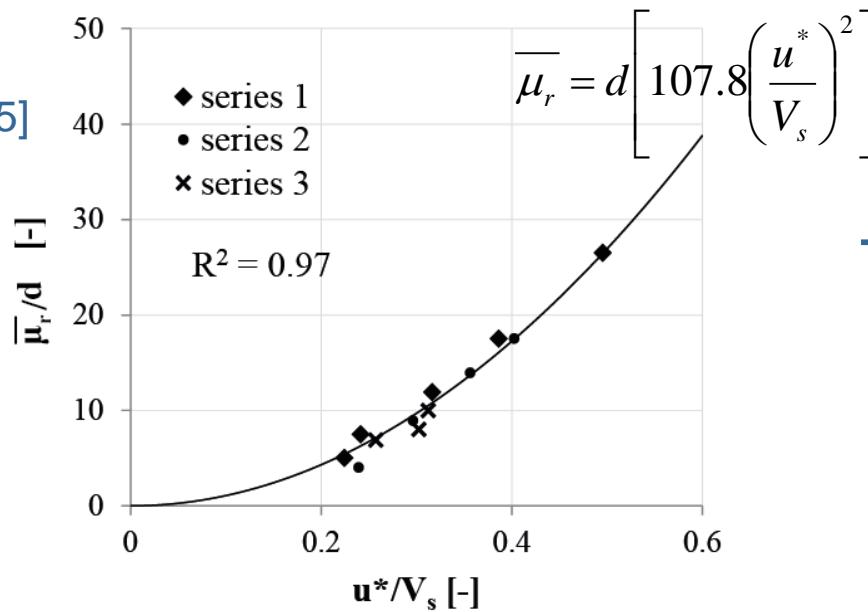
$$(1 - \lambda_p)L_a \frac{\partial f_T(x, t)}{\partial t} = D_T(x, t) - E_T(x, t)$$

$$(1 - \lambda_p) \frac{L_a}{E} \frac{\partial f_T(x, t)}{\partial t} = \int_0^{\infty} f_T\left(x - r, t - \frac{r}{v_p}\right) f_S(r) dr - f_T(x, t)$$



Step length → probability density function $f_s(r)$ depending on:

FLOW
INTENSITY^[5]



+ BED ROUGHNESS

$$\left(\frac{d}{\sigma_b} \right)^2$$

u^* =shear velocity
 V_s =settling velocity
 σ_b =standard deviation of bed elevation

$f_s(r)$ lognormally distributed

$$\begin{cases} \mu_r = \bar{\mu}_r \left(\frac{d}{\sigma_b} \right)^2 \\ \sigma_r = k \mu_r \end{cases}$$



Step length → comparison with other database:

Experiments 2009 [6]:

- Uniform bed $d_{50} = 5 \text{ mm}$
- Increasing $\tau^* = 0.06-0.09$
- Measured step length over $220 \times 80 \text{ mm} \rightarrow$ truncated pdf

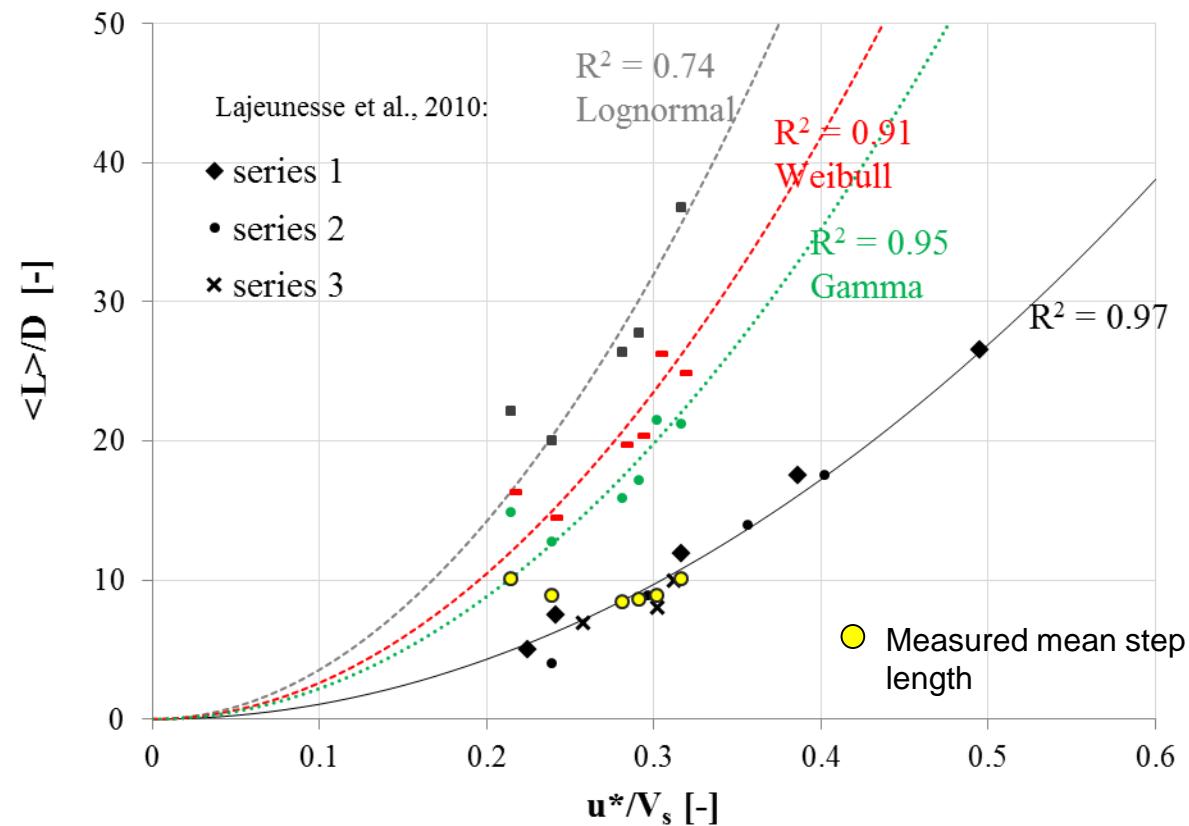
$$\bar{\mu}_r = d \left[k \left(\frac{u^*}{V_s} \right)^2 \right]$$

Lajeunesse: $k = 107$

Gamma fit: $k = 220.4$

Weibull fit: $k = 261.7$

Lognormal: $k = 355.8$





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Unpublished data representative of a medium river gravel at a reasonable transport rate^[7]:

- Sediment bed: $d_{16} = 0.13 \text{ mm}$, $d_{84} = 4.8 \text{ mm}$, $\sigma_s = 1.9 \text{ mm}$
- Tracer size: $d_T = 0.8 \text{ mm}$
- $u^* = 0.06 \text{ m/s}$, $\tau = 3.5 \text{ N/m}^2$, $u^*/u_c^* = 2.15$
- $L_a = 1.62 \sigma_b$ ^[8]
- Volumetric entrainment rate E ^[9]

$$W_i^* = \frac{(s-1)gq_{bi}}{F_i(u^*)^3}$$

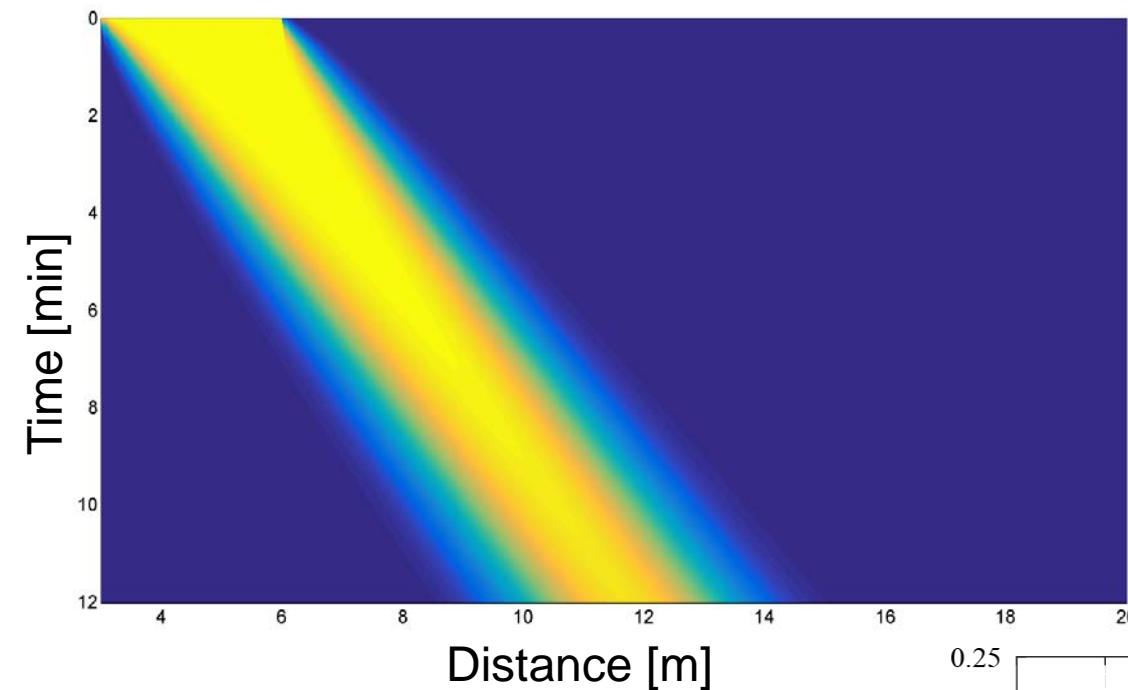
- v_p = particle's velocity at $z = 0.5d$ ^[10]

[7] Marion, Ph.D. thesis, Univ. of Padua, Italy, 1995.

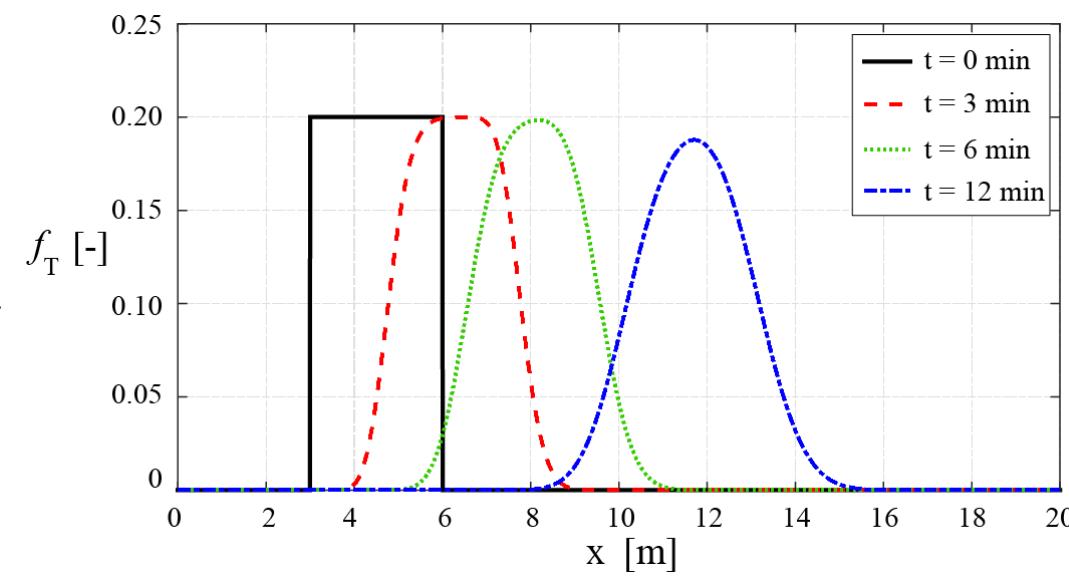
[8] Wong et al., Water Resour. Res, 2007.

[9] Wilcock and Kenworthy, Water Resour. Res, 2002.

[10] Nikora et al., J. Hydraul. Eng., 2001



Snapshots after
3, 6 & 12 minutes



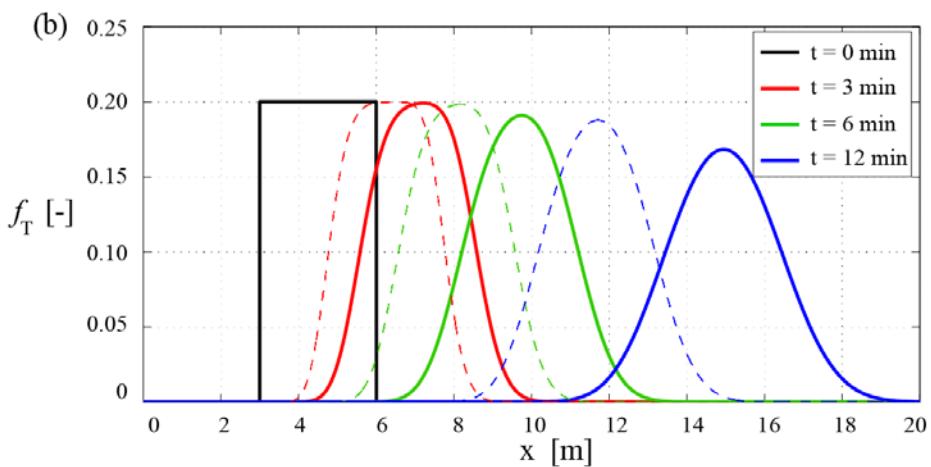
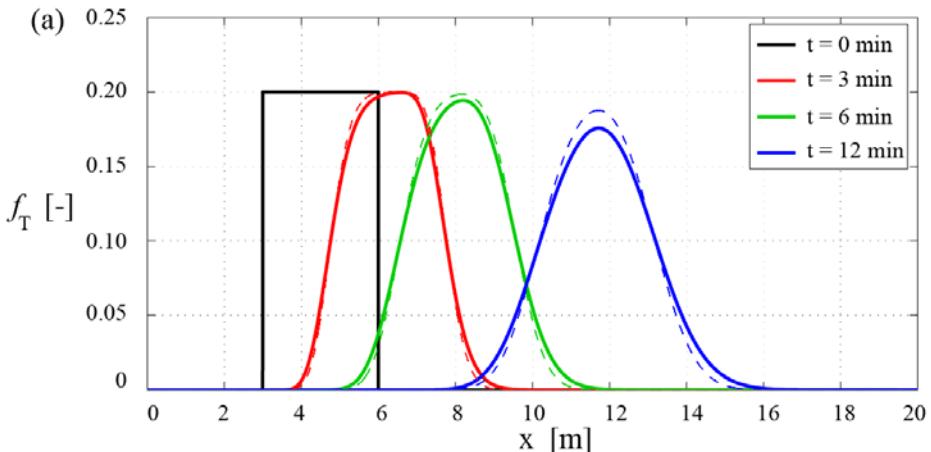


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Influence of particle's trajectory and flow intensity



$$\sigma_r = k \mu_r$$
$$k = 0.4 \rightarrow 0.8$$

u^* NEW 10% greater

Influence on:

- step length statistics
- entrainment rate

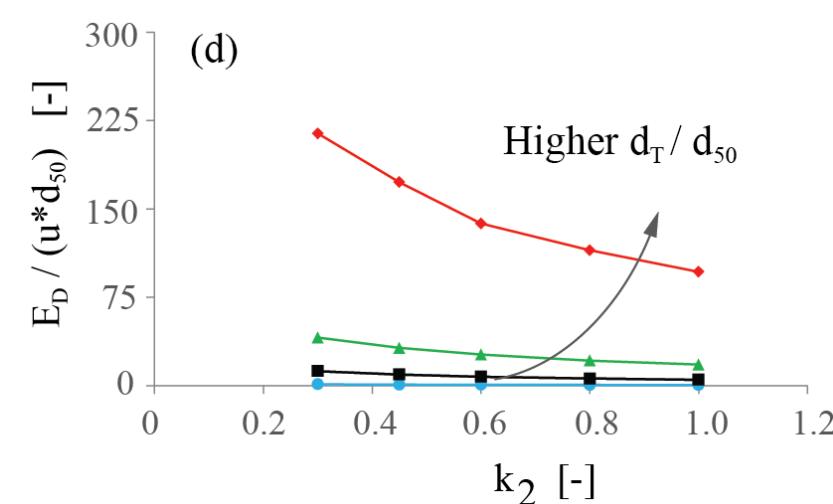
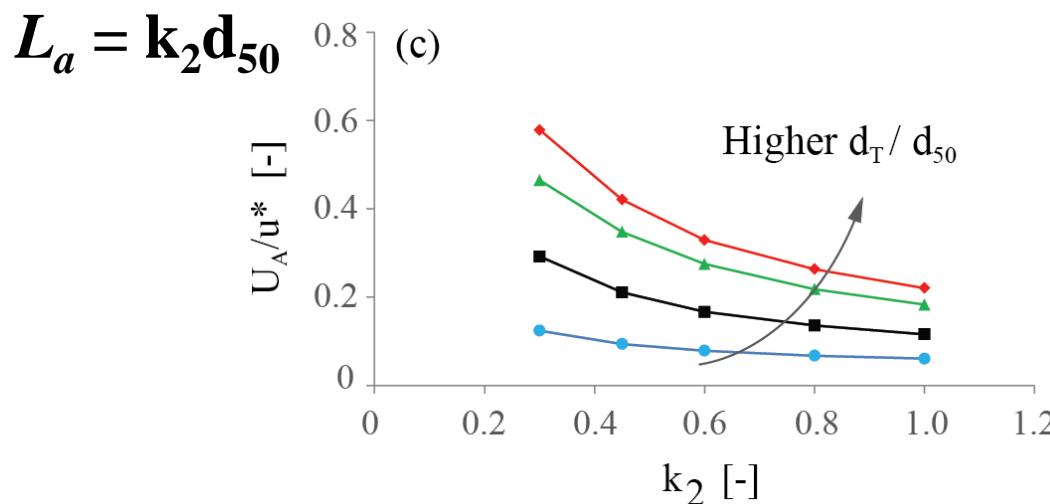
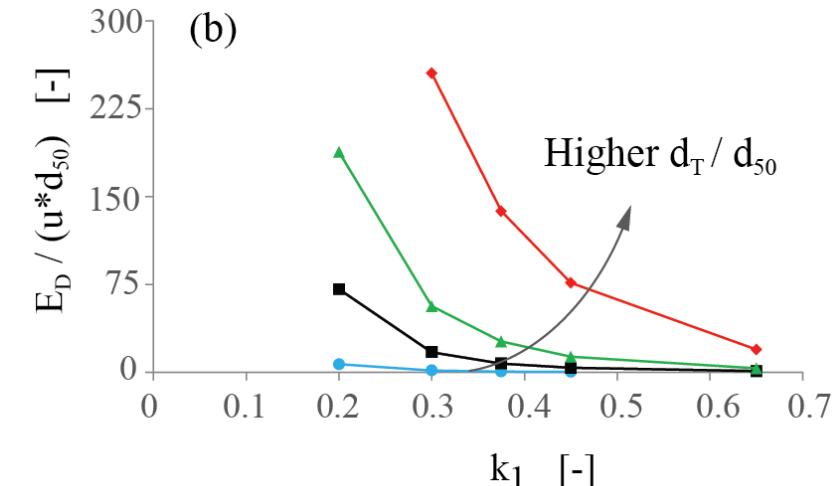
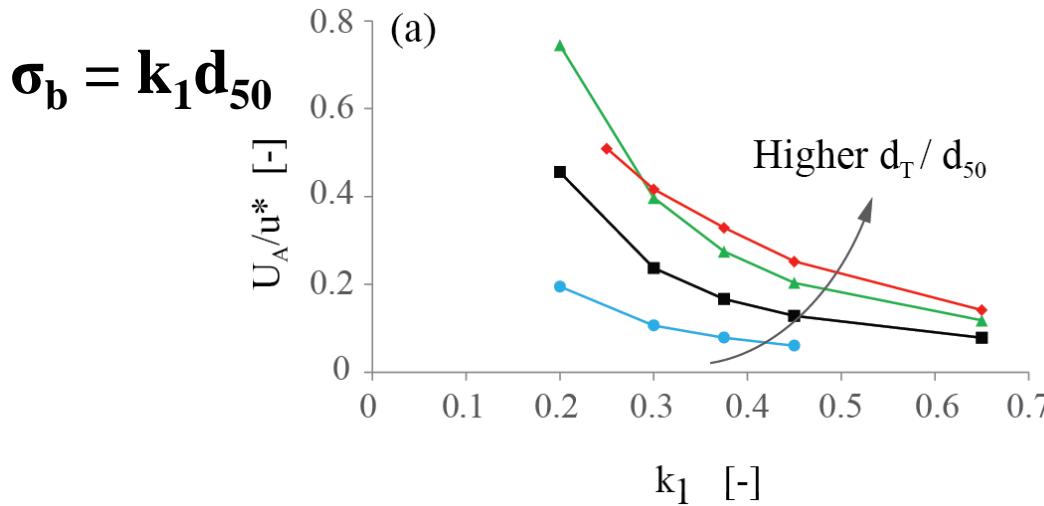


Enhanced mobility

	$d_{50} [\text{mm}]$	$d_T [\text{mm}]$	$u^*/u^*_c [-]$	$E [\text{m}^3/\text{m}^2\text{s}]$	$v_p [\text{m/s}]$	$k [-]$	$U_A/u^* [-]$	$E_D/u^* d_{50} [-]$
Original (Fig. 2)	1	0.8	2.15	$3*10^{-5}$	0.17	0.4	0.17	75
k case (Fig. 3a)	1	0.8	2.15	$3*10^{-5}$	0.17	0.8	0.17	110
u^* case (Fig. 3b)	1	0.8	2.40	$4.5*10^{-5}$	0.19	0.4	0.22	119

Influence of bed configuration

Advection and diffusion studied for $d_T/d_{50} = 0.4; 0.8; 1.2; 2.5$





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- Model (considering step length) performs the expected advection and diffusion motion of bedload particles
- Flow intensity, u^* , greatly affects mobility
- bed configuration = driving factor on sediment transport
 - Bed roughness, σ_b
 - Active layer, L_a

—————> Negative correlation on particles' mobility,
more significant for $d_T \geq d_{50}$
- Active layer acts as storage → diffusion for longer time scales

Future investigations: bed roughness role in diffusive processes under longer time scales



Acknowledgement to Project “HYTECH: Hydrodynamic Transport in Ecologically Critical Heterogeneous Interfaces”

Thanks for
your
attention