

# Interaction between storm water conduit flow and overland flow for numerical modelling of urban area inundation

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# Urban flooding

## Pluvial (surface) flooding.

Caused by extreme rainfall events that cannot be absorbed by drainage system.

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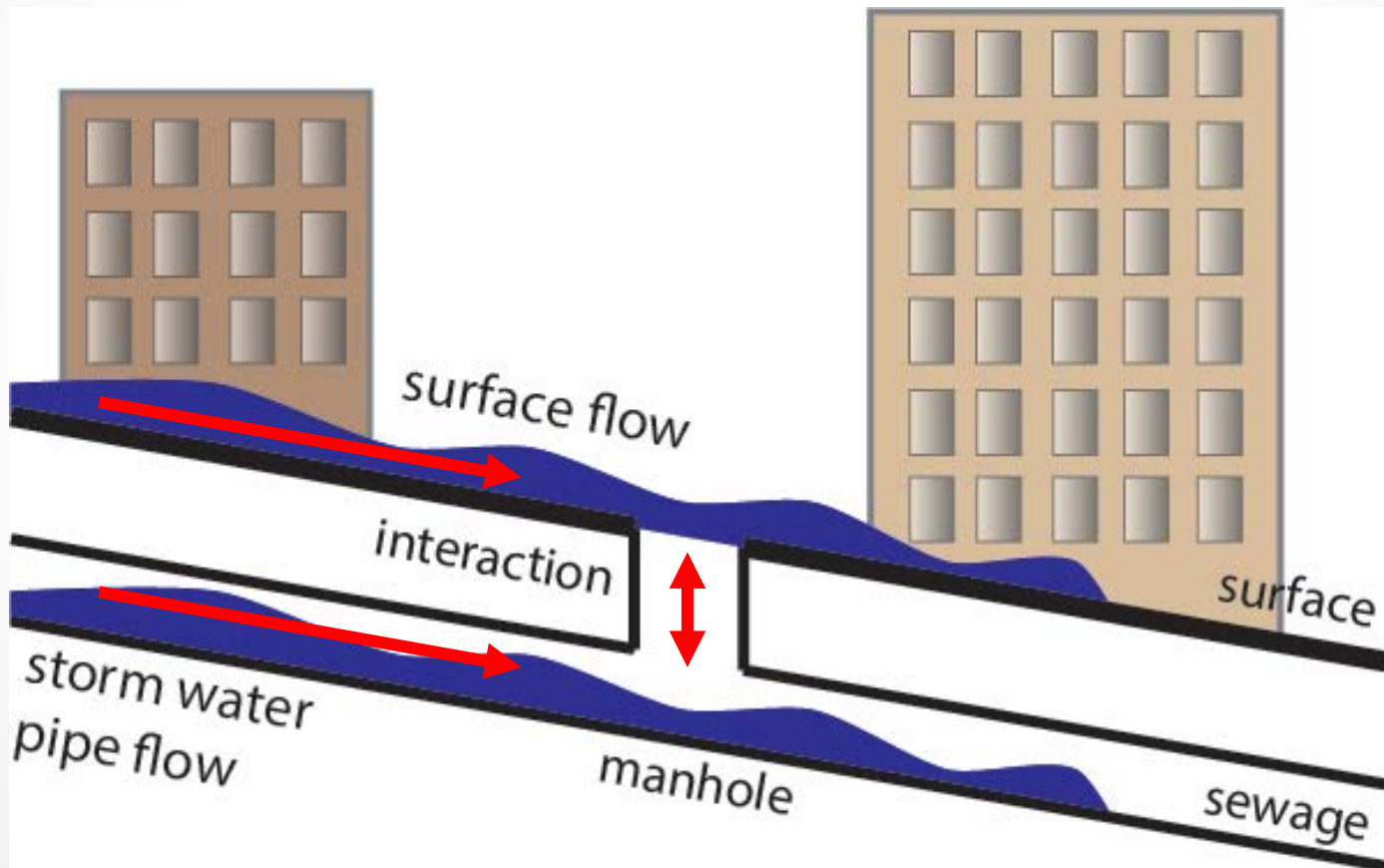
source: „[www.trojmiasto.pl](http://www.trojmiasto.pl)”



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# Modelling of urban flooding

Dual drainage modelling (1D+2D):



# Storm water pipe flow (1D)

## The Saint-Venant Equations

$$\frac{\partial \mathbf{U}}{\partial t} + \frac{\partial \mathbf{F}}{\partial x} = \mathbf{S}$$

$$\mathbf{U} = \begin{pmatrix} A \\ Q \end{pmatrix} \quad \mathbf{F} = \begin{pmatrix} Q \\ Q^2 / A + I \end{pmatrix} \quad \mathbf{S} = \begin{pmatrix} 0 \\ gA(S_o - S_f) \end{pmatrix}$$

# Storm water pipe flow (1D)

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Solving:

- finite differences method,
- finite elements method.

## Surface flow (2D)

### Shallow Water Equations (SWE)

$$\frac{\partial \mathbf{U}}{\partial t} + \frac{\partial \mathbf{E}}{\partial x} + \frac{\partial \mathbf{G}}{\partial y} + \mathbf{S} = 0$$

$$\mathbf{U} = \begin{pmatrix} h \\ uh \\ vh \end{pmatrix}, \quad \mathbf{S} = \begin{pmatrix} 0 \\ -gh(S_{ox} - S_{fx}) \\ -gh(S_{oy} - S_{fy}) \end{pmatrix}, \quad \mathbf{E} = \begin{pmatrix} uh \\ u^2h + 0.5gh^2 \\ uvh \end{pmatrix}, \quad \mathbf{G} = \begin{pmatrix} vh \\ uvh \\ v^2h + 0.5gh^2 \end{pmatrix}$$

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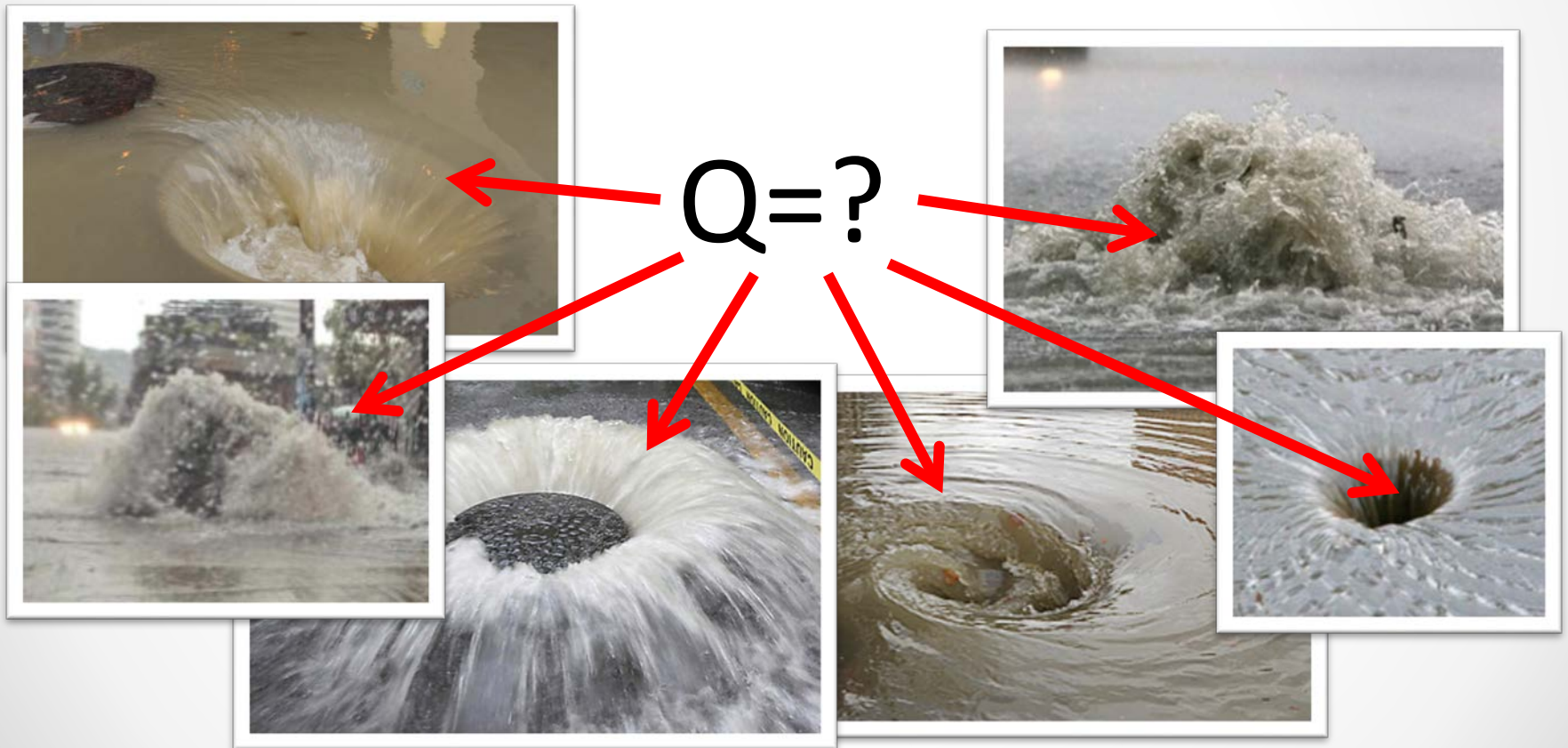
Solving:

- finite volume method,
- finite element method,
- finite difference method.



# Concept of dual drainage modelling

Problem of interaction!



# Quasi coupled models

Modelling made in steps

Storm water  
pipe flow  
modelling

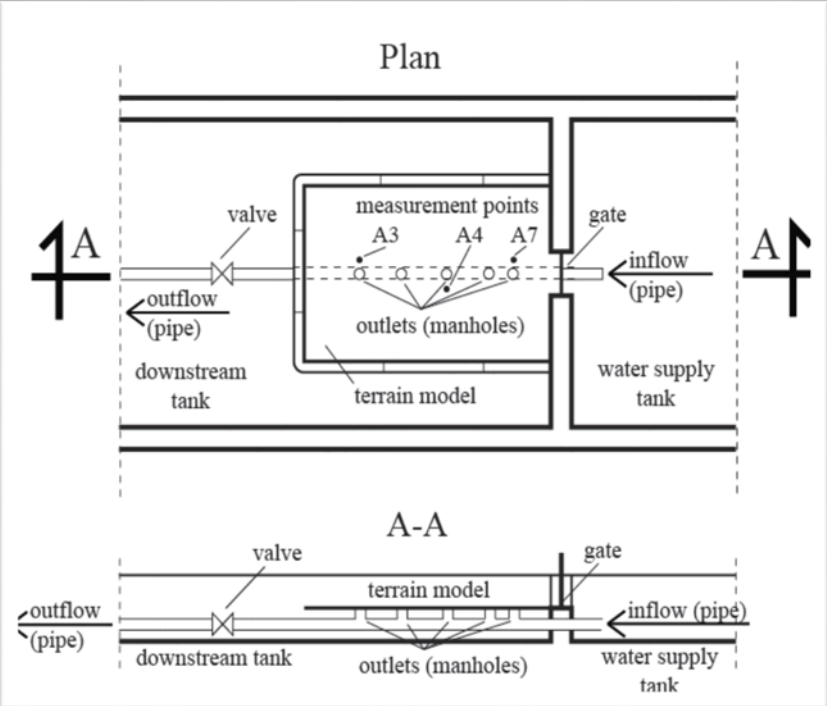
Results  
(piezometric  
pressure)

Surface flow  
modelling  
(step 1 results  
taken as data)

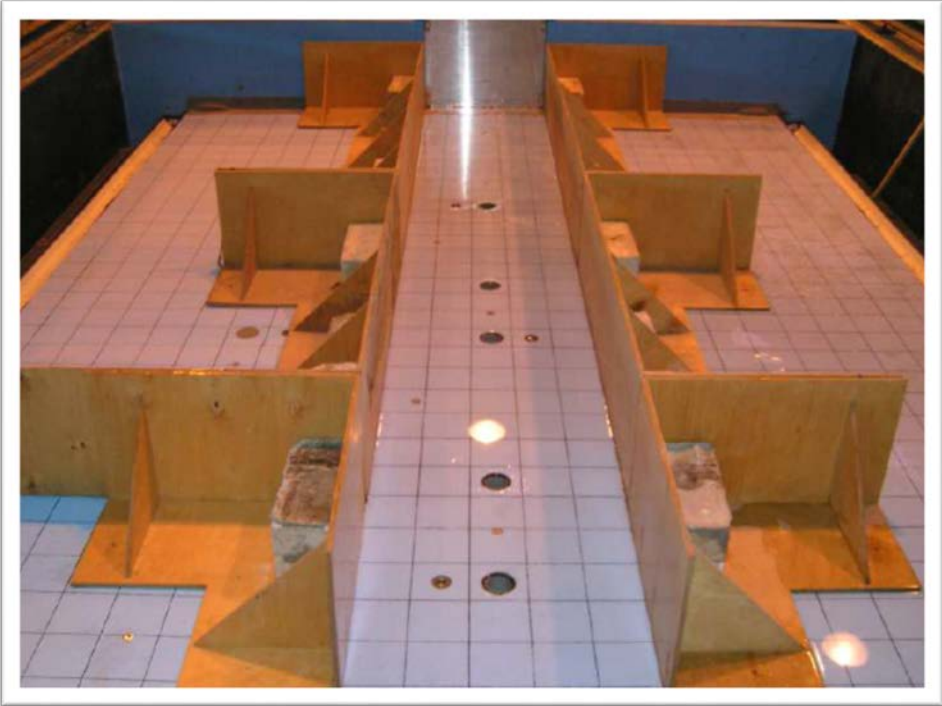
Results  
(Surface water  
depth)

# Quasi coupled models

Test held at hydraulic laboratory of the Gdańsk University of Technology



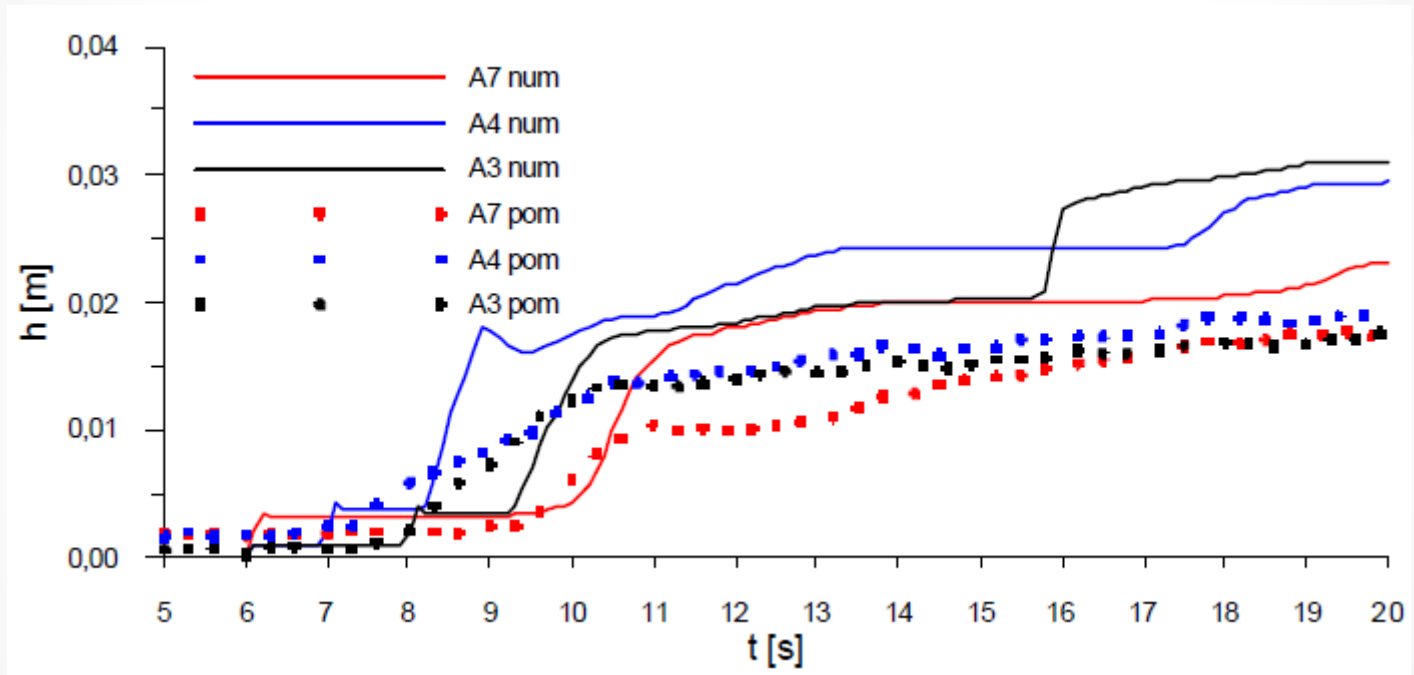
Scheme of urban area inundation laboratory stand



Photography of urban area inundation laboratory stand

# Quasi coupled models

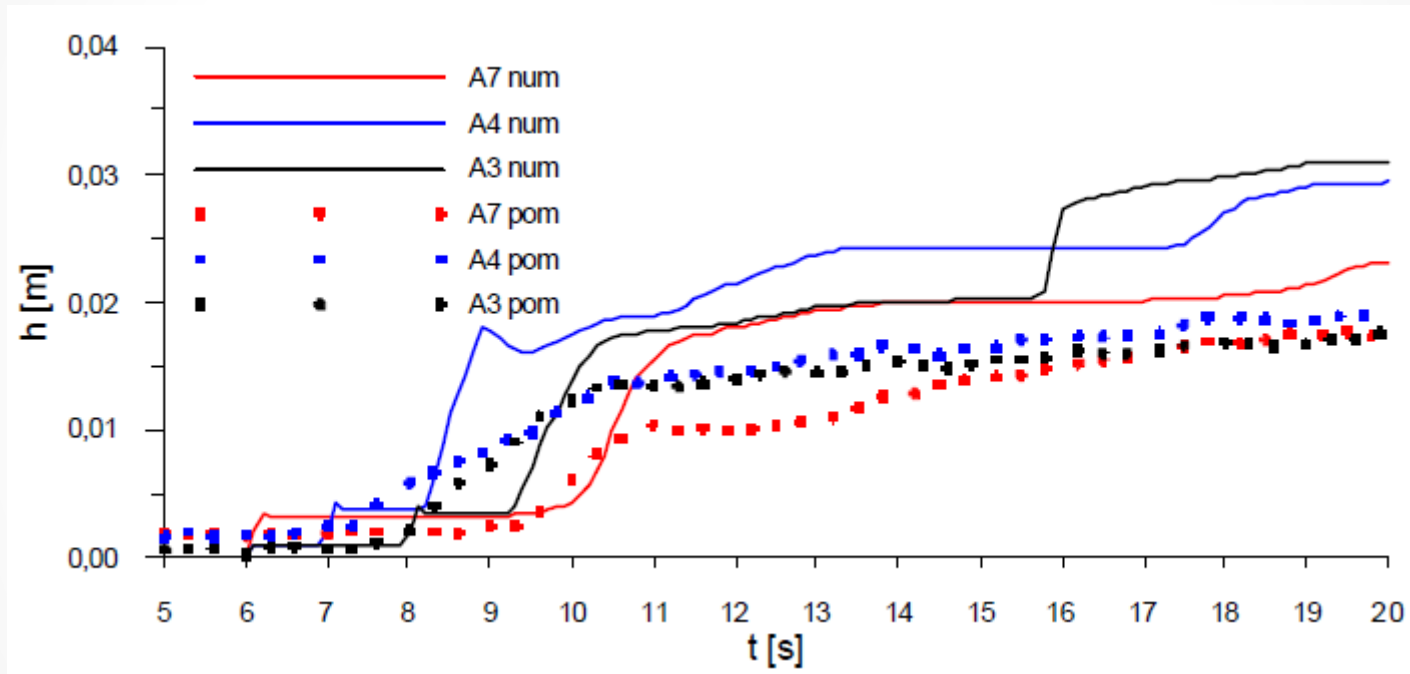
## Results



Comparison of measurements and calculations for urban area inundation experiment

# Quasi coupled models

## Results



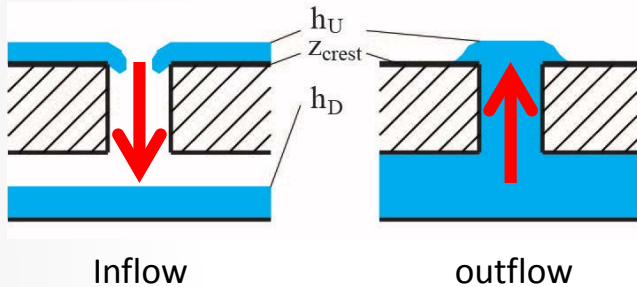
Comparison of measurements and calculations for urban area inundation experiment

Conclusion: interaction is needed in calculations

# Integrated models

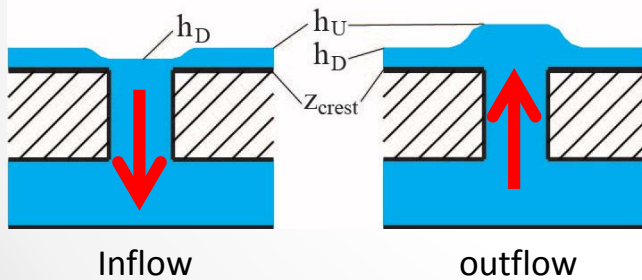
## Interaction between pipe and surface flow

Free weir:



$$Q = \text{sign}[h_{mh} - h_{2d}] c_w w \sqrt{2g} (h_U - z_{crest})^{3/2}$$

Submerged weir and orifice:



$$(h_U - z_{crest}) < A_{mh}/w$$

$$Q = \text{sign}[h_{mh} - h_{2d}] c_w w \sqrt{2g} (h_U - z_{crest}) (h_U - h_D)^{1/2}$$

$$(h_U - z_{crest}) \geq A_{mh}/w$$

$$Q = \text{sign}[h_{mh} - h_{2d}] c_o A_{mh} \sqrt{2g} (h_U - h_D)^{1/2}$$

# Interaction

Questions and problems?

No verification presented (why we need it?):

# Interaction

Questions and problems?

No verification presented (why we need it?):

Verification if equations are valid for:

- high surface slopes,
- high values of velocity,

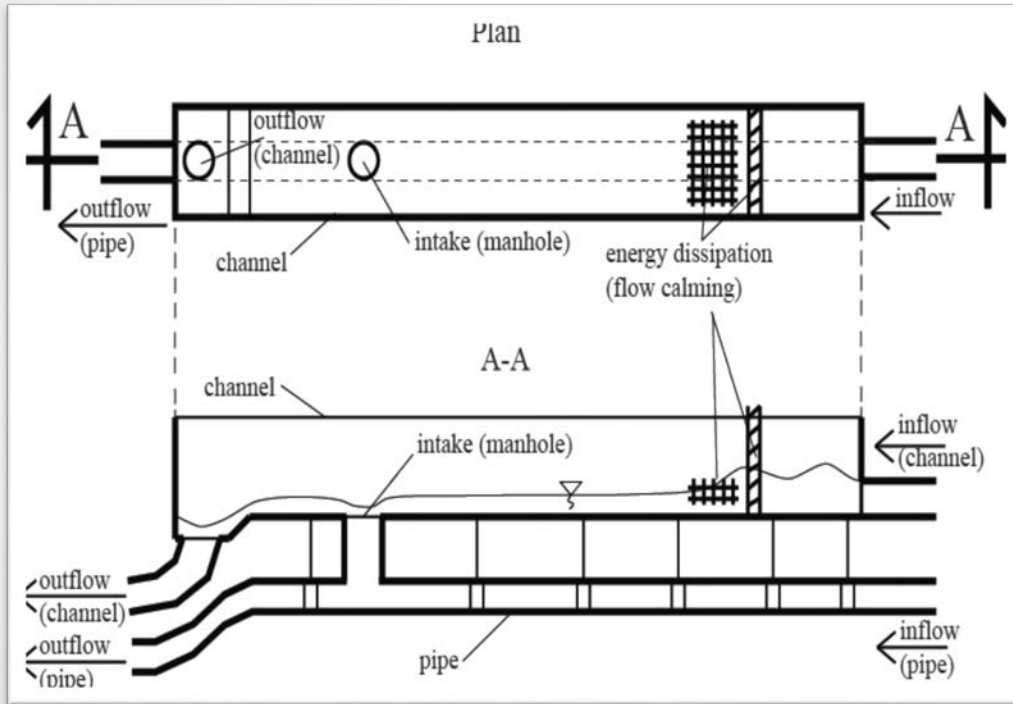
Is there a need to include velocity in calculations?

How can we do that?



# Experiment to verify interaction formulas

## Test stand



Scheme of laboratory stand for surface and sewage flow interaction experiments

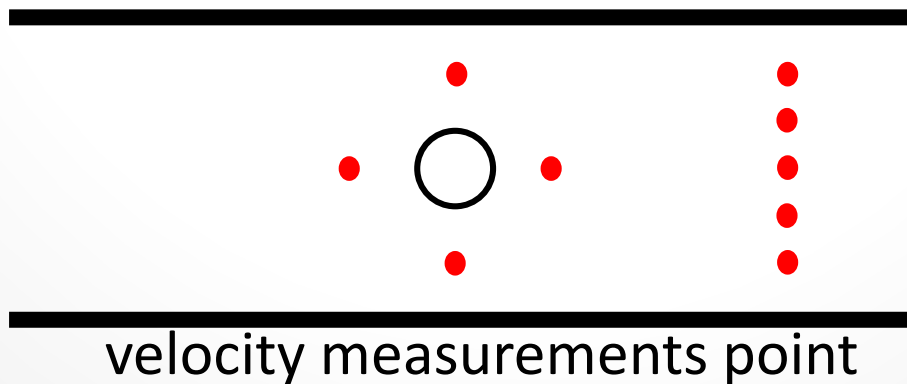
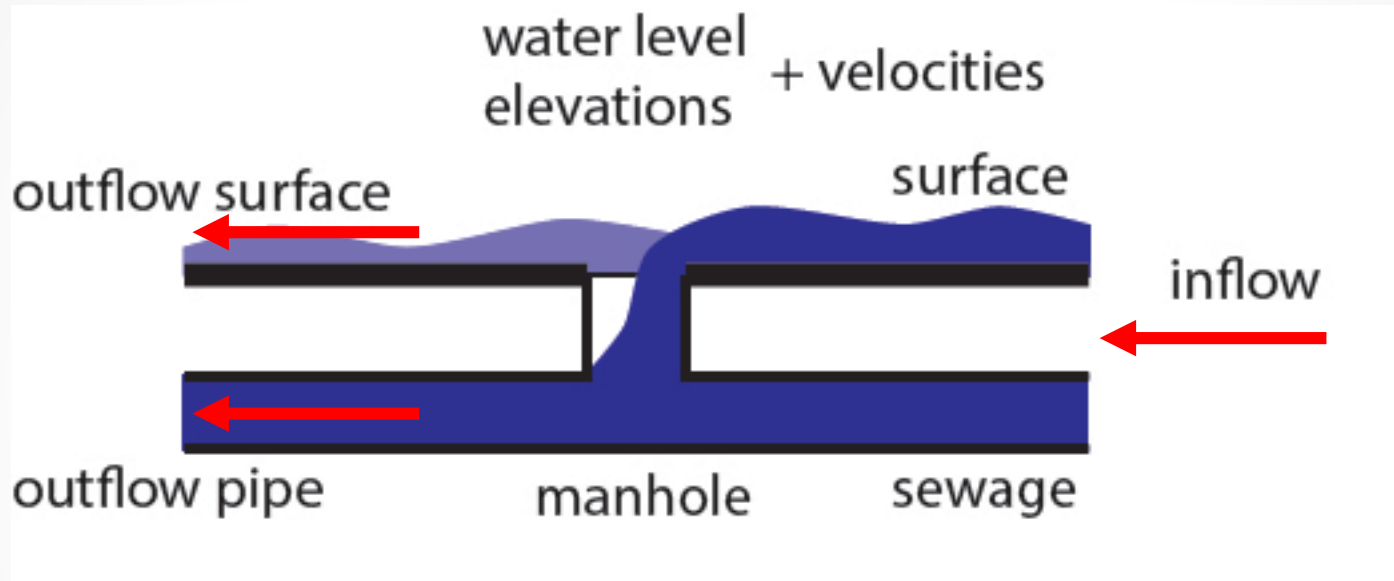
channel:  
length: 5.5m  
width: 0.4m  
pipe:  
diameter: 110mm

Photography of laboratory stand for surface and sewage flow interaction experiments



# Experiment to verify interaction formulas

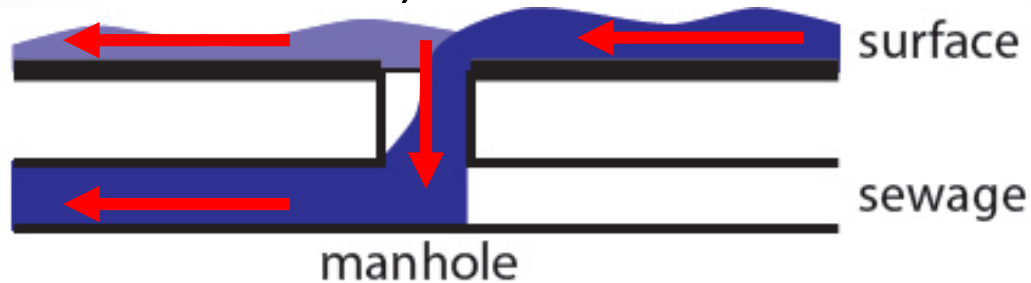
## Test stand - measurements



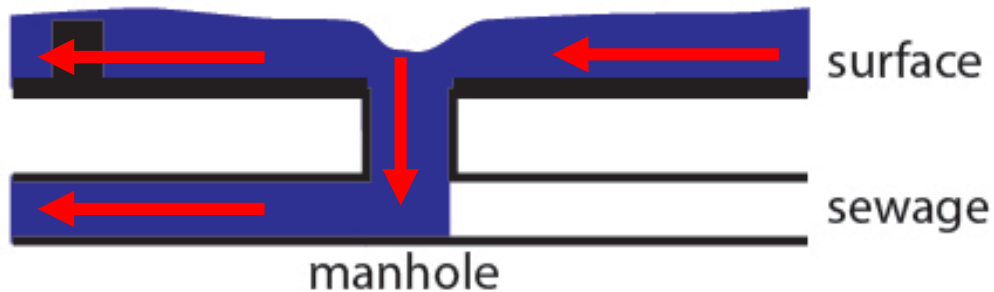
# Experiment to verify interaction formulas

## Test scenarios

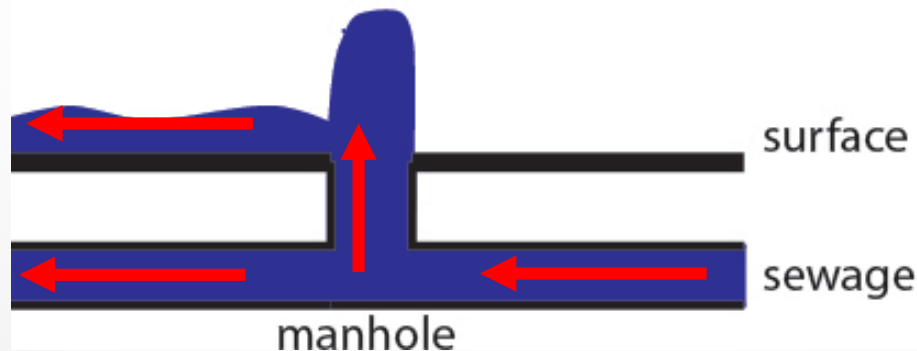
- inflow from surface,



- Inflow from surface with increased water depth,



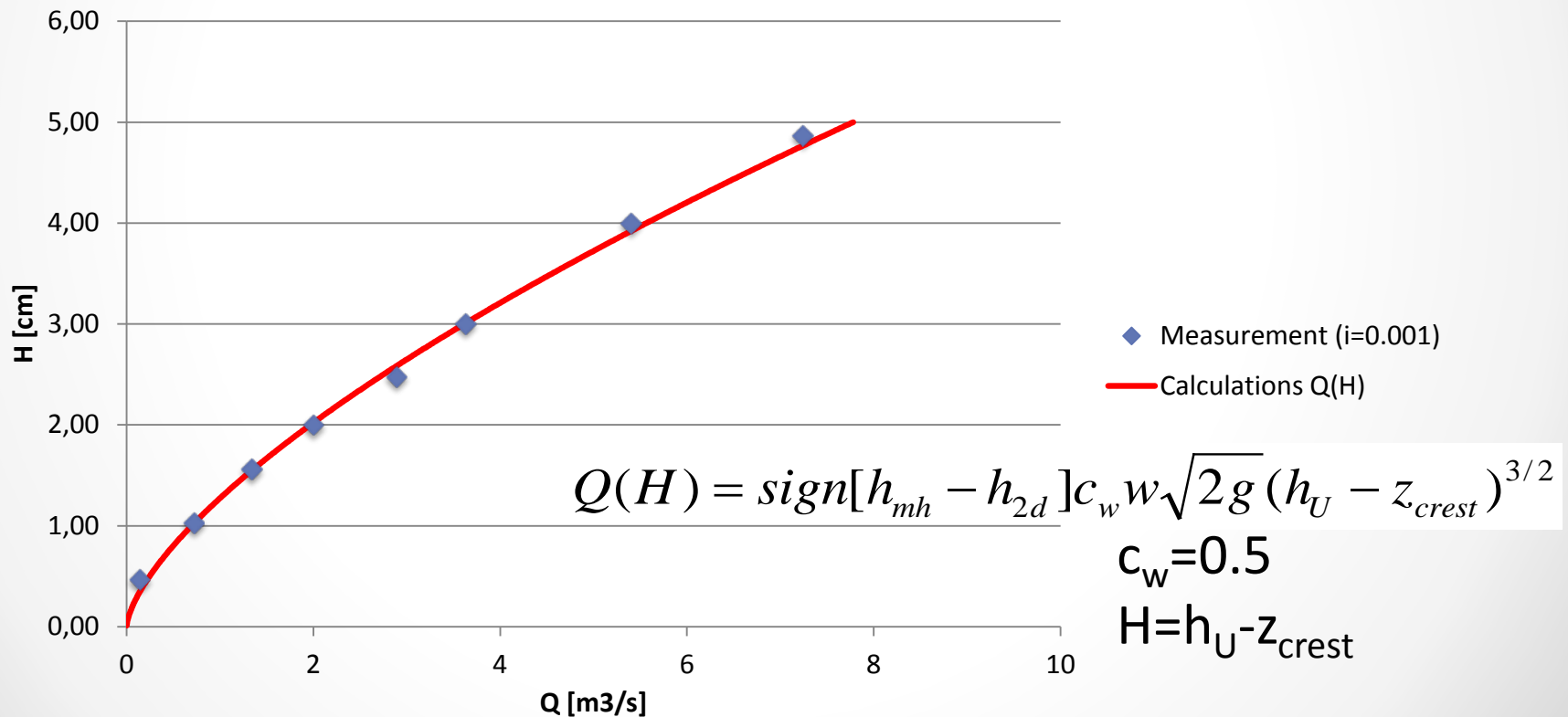
- free inflow from sewage,



# Experiment to verify interaction formulas

## Results

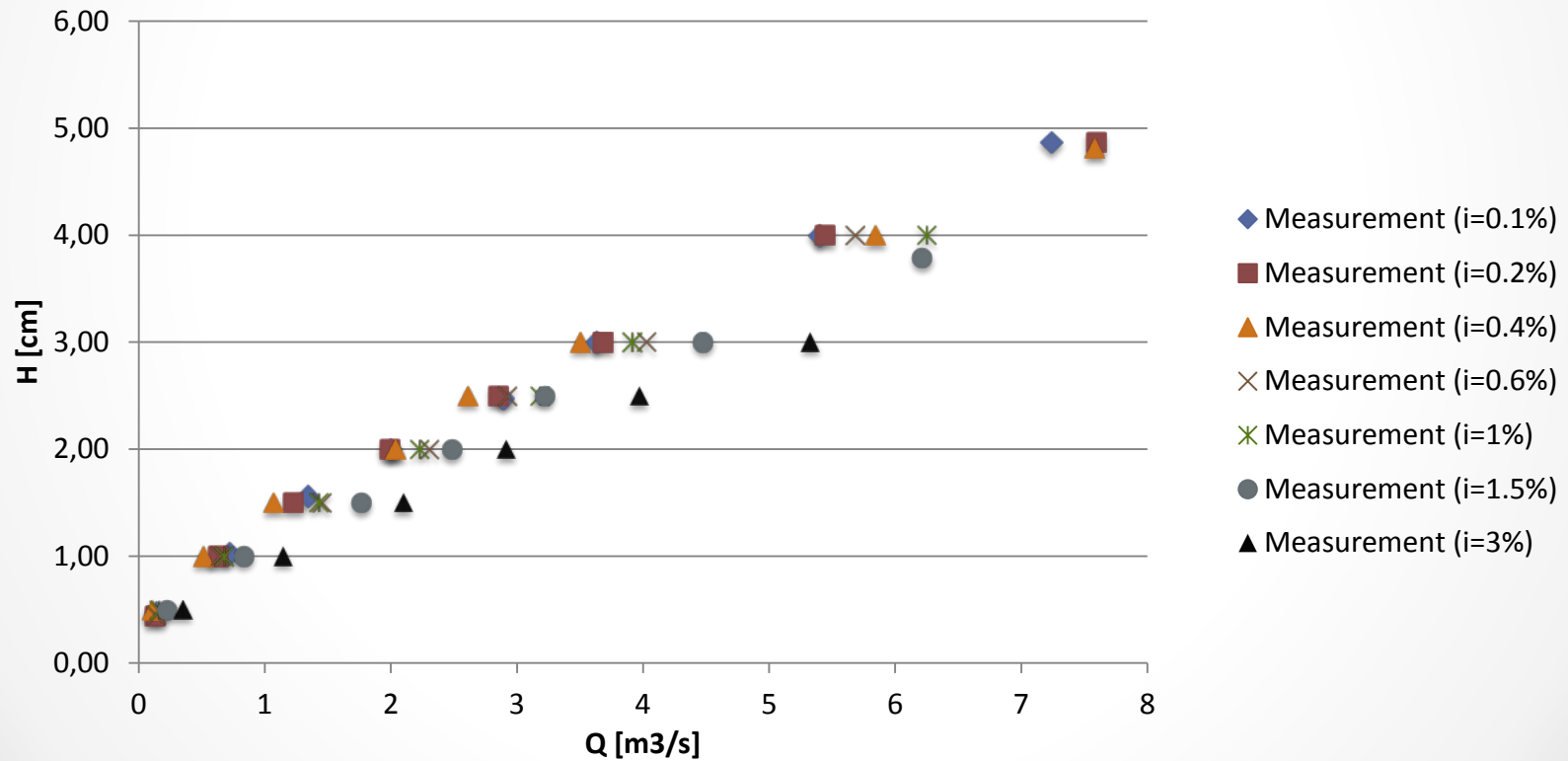
Verifying formula for free weir with 1‰ surface slope:



# Experiment to verify interaction formulas

## Results

Comparing inflow values for different slope:



## Experiment to verify interaction formulas

First approach based on elevation and velocity:

Basic free weir formula:

$$Q = \text{sign}[h_{mh} - h_{2d}] c_w w \sqrt{2g} (h_U - z_{crest})^{3/2}$$

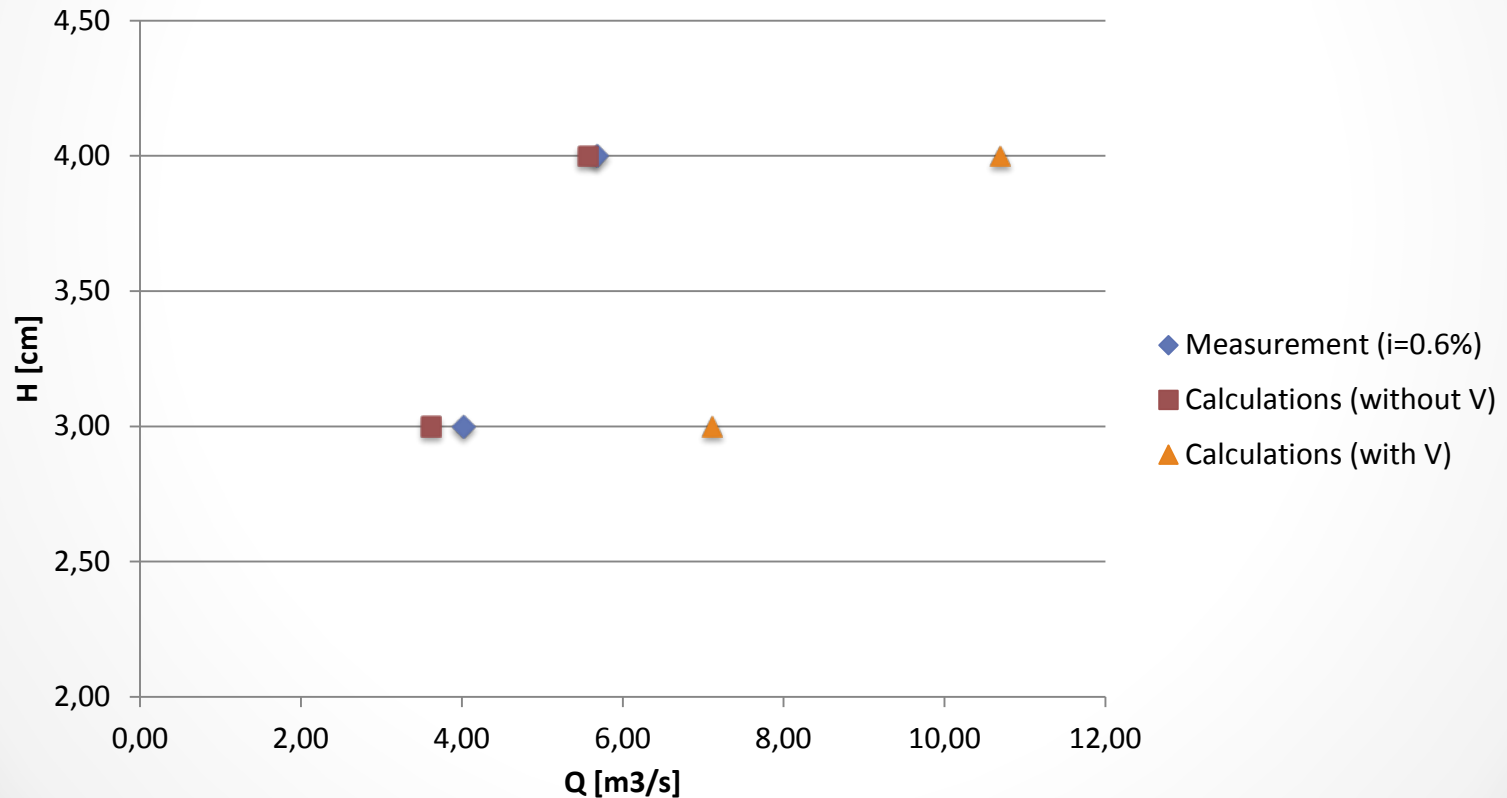
Free weir formula with velocity head:

$$Q = \text{sign}[h_{mh} - h_{2d}] c_w w \sqrt{2g} \left[ (h_U - z_{crest} + \frac{V^2}{2g})^{3/2} - (\frac{V^2}{2g})^{3/2} \right]$$

# Experiment to verify interaction formulas

First approach based on elevation and velocity:

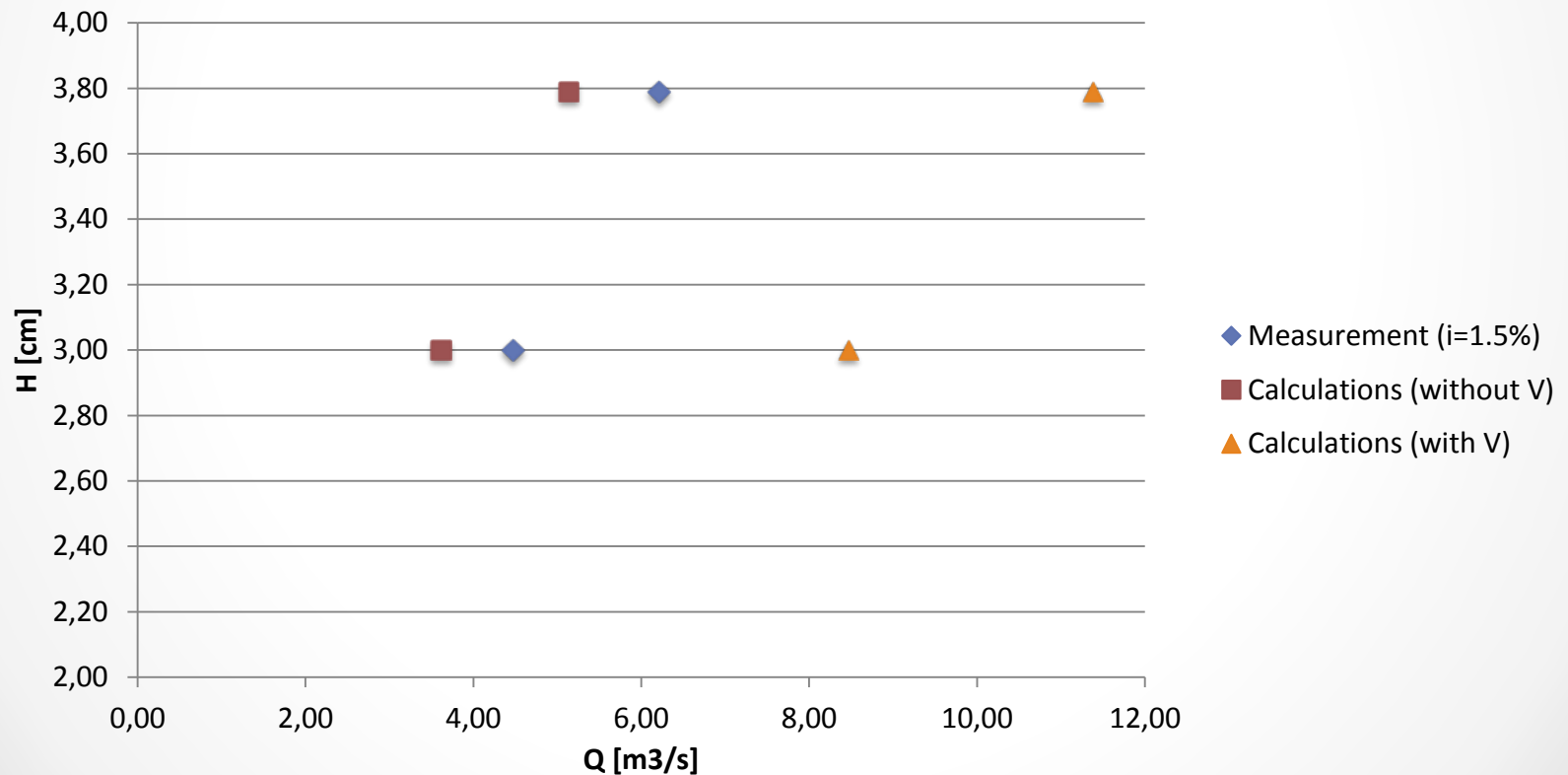
Results ( $i=0.6\%$ ):



# Experiment to verify interaction formulas

First approach based on elevation and velocity:

Results ( $i=1.5\%$ ):





## Experiment to verify interaction formulas

### Conclusion of current results:

- for not sloped channels basic free weir formula is correct,
- with slope increase we observe higher values of inflow into manhole,
- there is a need for including velocity values into equations,
- approach using basic hydraulic formulas for free weir including velocity head is not suitable for this case.

Thank You  
for your attention