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UNIwersYTET
ROLNICZY
im. Hugona Kołłątaja
w Krakowie

Prediction of fluvial processes change
within mountain streams
on the example of
the Porębianka stream



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Presentation schedule

1. Basin description
2. Porębianka stream
3. Flow data
4. Methodology
5. Results
6. Results interpretation
7. Additional solutions
8. Conclusions

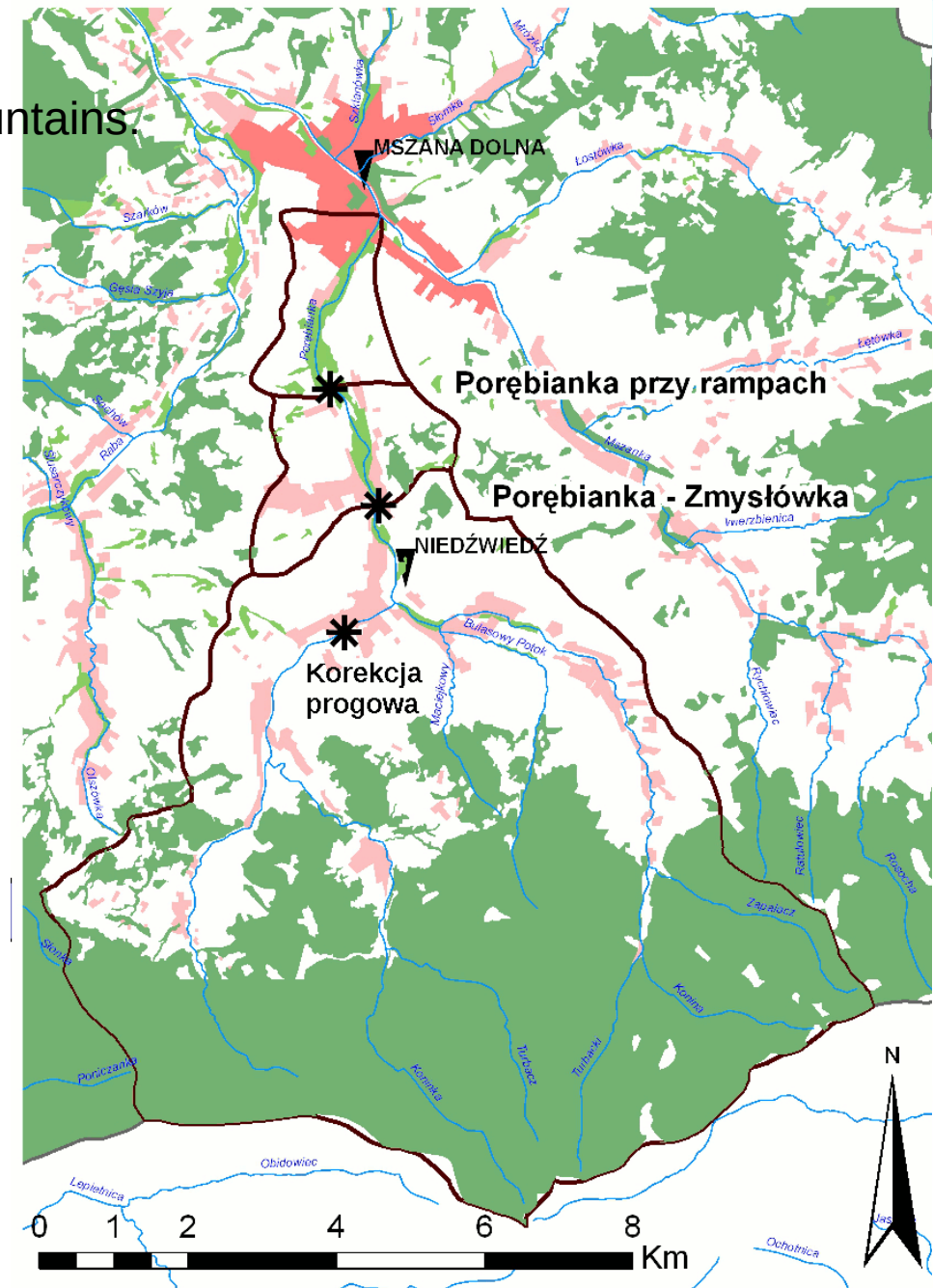
1. Basin

Porębianka is a stream flowing in the Gorce Mountains.
It is a left tributary of the Mszanka River.

Sources of the stream are located in Pośrednie col between Obidowiec and Bardo Mountains (1030 m a.s.l.).

**Area of basin is 71 km²
and the length – 15 km.**

It can be stated that afforestation had increased here from 49.2 % in year 1984 to 59% in 2003.



1. Basin

reaches

reach 1 - korekcja progowa

sub-basin1 $A1 = 33.6 \text{ km}^2$

slope (middle part) $S = 0.9\%$

slope (lower part) $S = 0.1\%$

reach 2 - Zmysłówka

sub-basin2 $A2 = 63.5 \text{ km}^2$

slope high water $S = 1.8\%$

slope low water

$S_r = 7.9\%$

$S_p = 0.8\%$

reach 3 – przy rampach

sub-basin3 $A3 = 67.7 \text{ km}^2$

slope high water $S = 1.9\%$

slope low water

$S_r = 6.0\%$

$S_p = 0.14\%$

basin

total area $A = 71 \text{ km}^2$

Niedźwiedź water gauge

sub-basin $A = 61.7 \text{ km}^2$



1. Basin



reach I - river is regulated by series of small steps

1. Basin



reach II - river banks are stabilized

1. Basin



reach III - river is regulated by ramps

2. Porebianka Stream



reach I, cross-section 1 - river is regulated by series of small steps

2. Porebianka Stream



reach I, cross-section 2 - river is regulated by series of small steps

2. Porebianka Stream



reach I, cross-section 2 - river is regulated by series of small steps

2. Porebianka Stream



reach I, cross-section 3 - river is regulated by series of small steps

2. Porębianka Stream



below reach I, bridge in Niedźwiedź – rocky bed

2. Porebianka Stream



below the outlet of the Konina Stream

2. Porębianka Stream



reach II (Zmysłówka) - river banks are stabilized

2. Porębianka Stream



reach II (Zmysłówka) - river banks are stabilized

2. Porebianka Stream



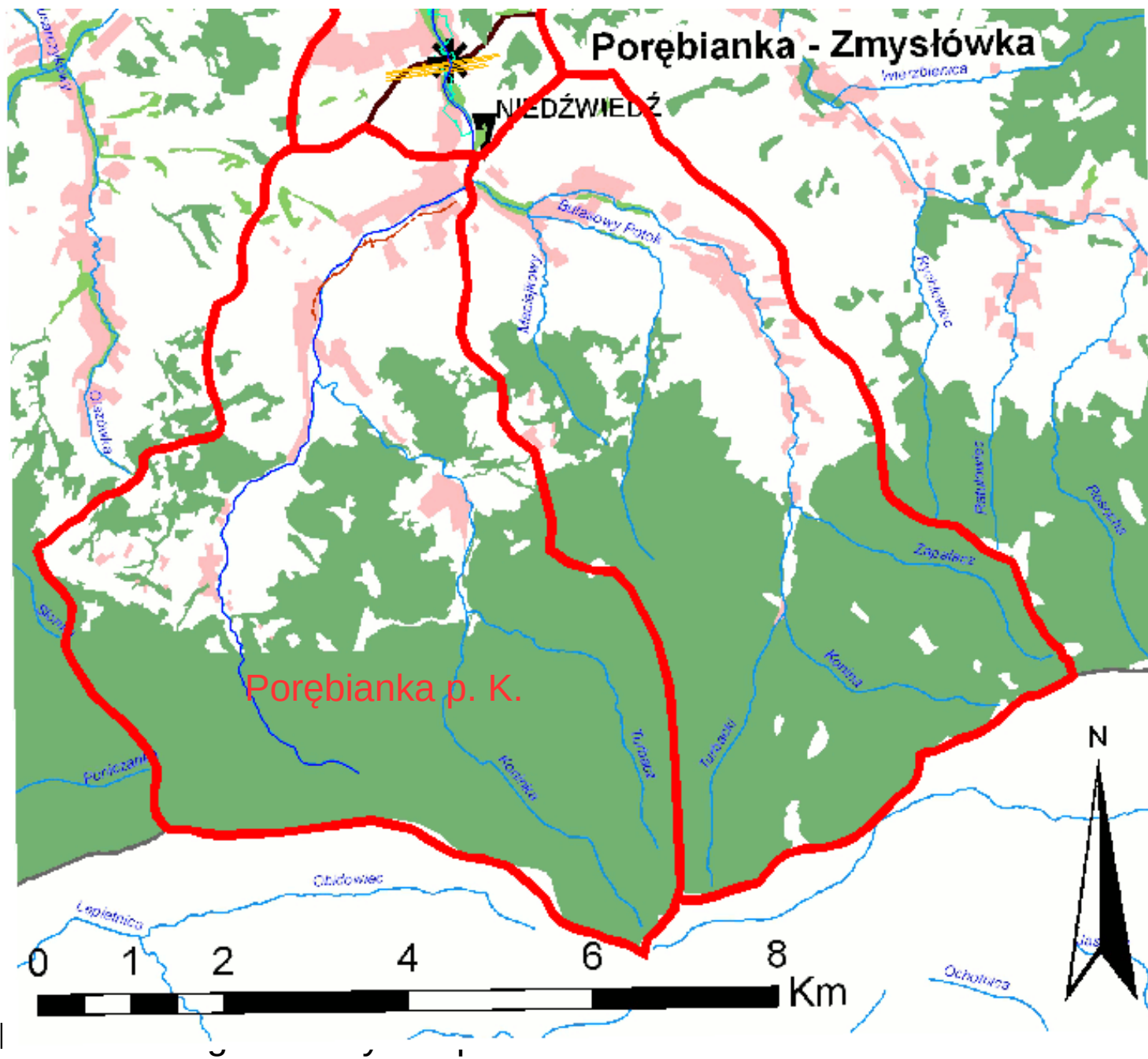
reach III - river is regulated by ramps

2. Porebianka Stream



reach III - river is regulated by ramps

2. Porębianka Stream



reach II

3. Flow data

p%	Niedzwiedź water gauge	reach I	reach II	reach III
	Q [m ³ /s]			
0.1	273	149	277	288
0.2	245	134	249	259
0.3	229	125	233	242
0.5	209	114	212	221
1	181	99	184	191
2	153	83	155	162
3	136	74	138	144
4	124	68	126	132
5	115	63	117	122
10	85.7	47	87.1	91.2
20	56.7	31	57.7	60.6
25	47.5	26	48.4	50.9
30	40.4	22	41.2	43.3
40	30.1	16	30.8	32.5
50	25	14	25.6	27

source: IMGW, calculated with Punzet equation

probability of the maximum annual peak discharges

4. Methodology

1. In situ measurements – 2009 – 2014

- geodesy measurement (long. profiles, cross-sections)
- granulometry
- photos

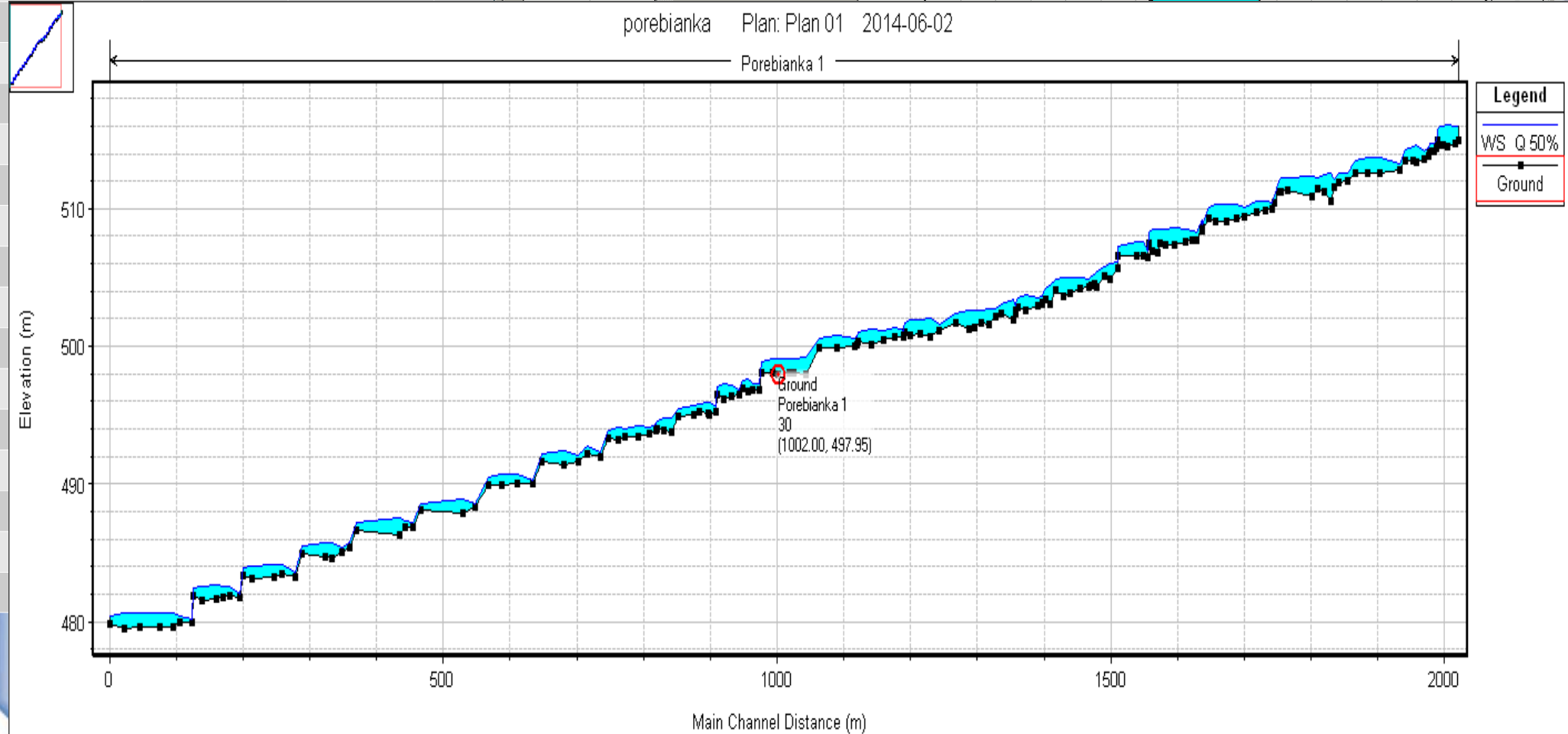
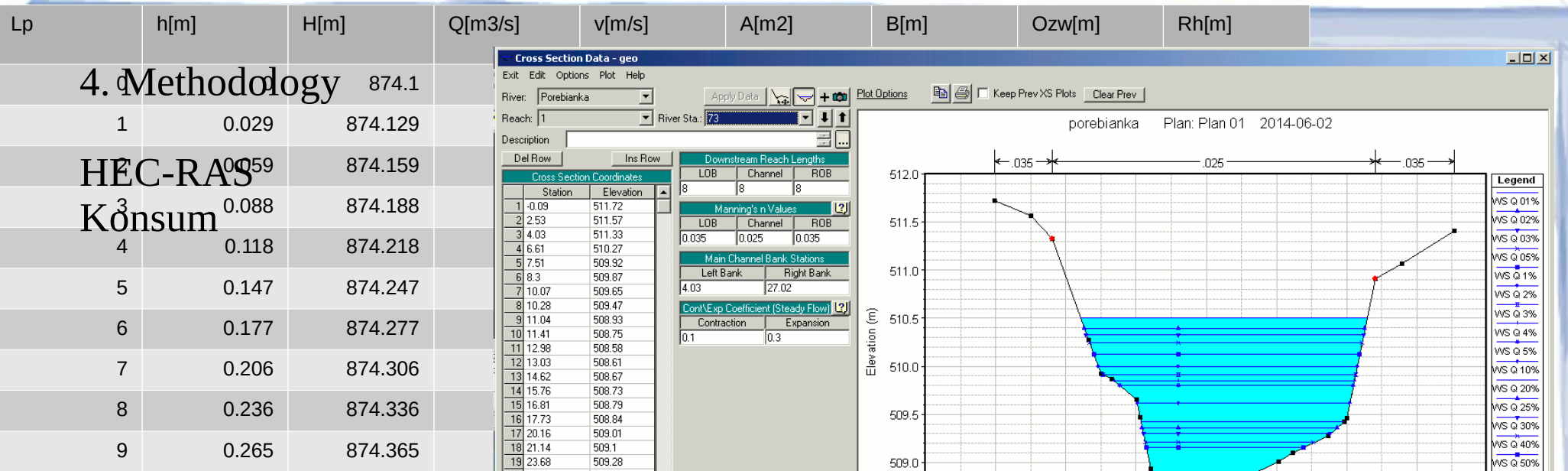
Assumption – steady state

2. HEC-RAS

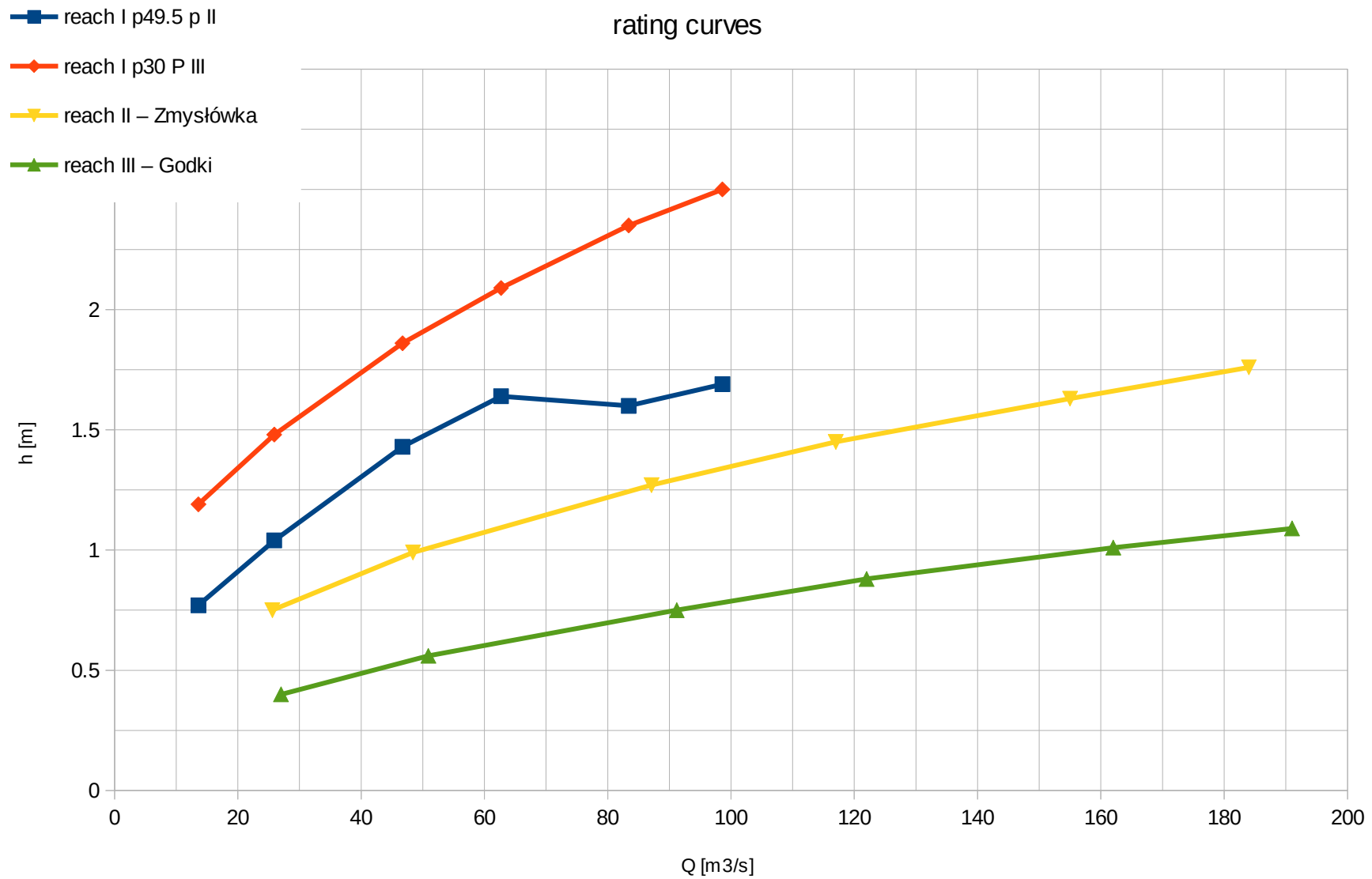
3. Konsum, Chezy – Manning eq.

4. Armour, Wang, Bartnik, Gessler, Komura eq-s

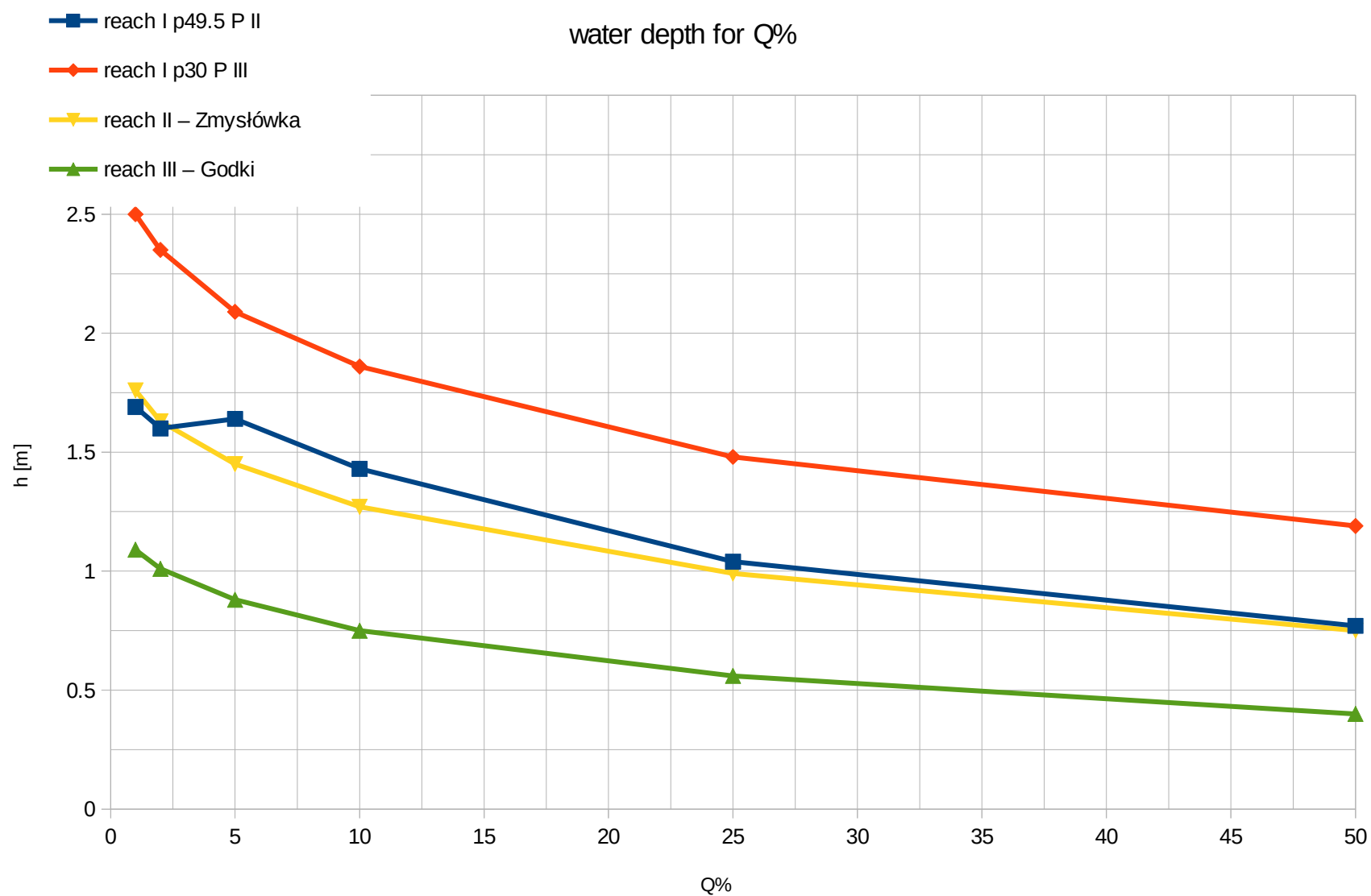
5. Incipient motion – bed stability prognosis



4. Methodology



4. Methodology



4. Methodology

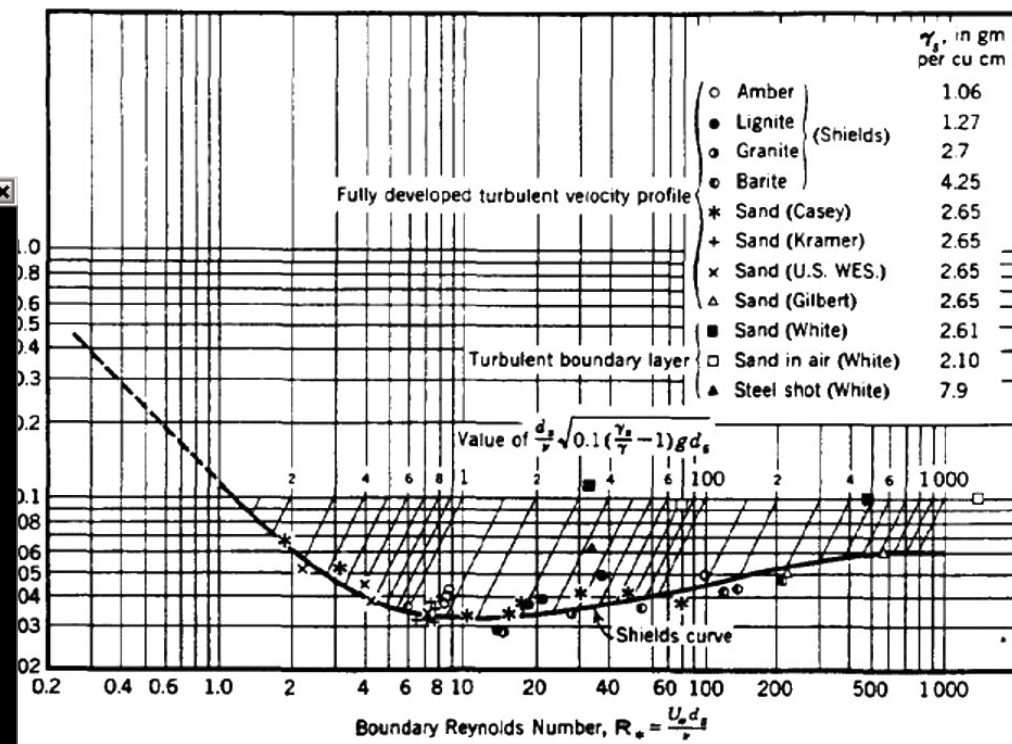
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all7a.win.exe

University of Agriculture in Krakow
Faculty of Environmental Engineering and Land Surveying
Department Water Engineering and Geotechnics
Krakow Al. Mickiewicza 24/28

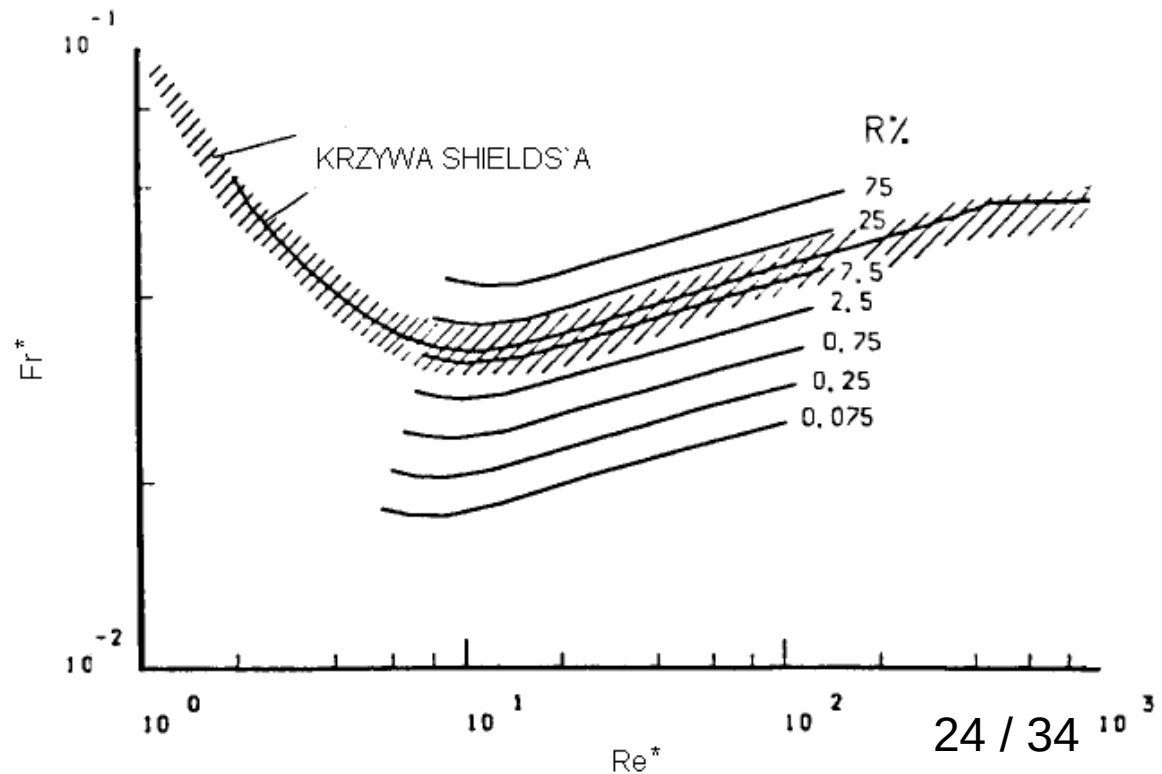
By : Andrzej Struzynski
Version 1.17 - Krakow VI 2013

AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA
o A. Struzynski - rmstruzy@cyf-kr.edu.plo
max. number of cross-section points: v.DOS - 520, v.WIN32 -
AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA
  
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$$p_a(d) = \frac{\sum_{d \min}^d q(d) \Delta p_0(d)}{\sum_{d \min}^d q(d) \Delta p_0(d)}$$

$$q_g = \phi_p \frac{\tau_{gr}}{\tau_0}$$



4. Methodology

$$\tau_c = f_i g \Delta \rho_s d_i$$

$$\varepsilon = f_i / f_m = \left(d_i / d_m \right)^{-r}$$

$$f_i = \frac{f_{m1}}{1.786 \left(\frac{d_i}{d_m} \right)^{0.947}}$$

$$f_i = \frac{f_{m2}}{\left(\frac{d_i}{d_m} \right)^{0.314}}$$

Wang [1977]

$$f_{m1} = 0.039 \rho^{0.26}$$

$$\text{for } d_i/d_m < 0.6$$

$$f_{m2} = 0.028 \rho^{0.26}$$

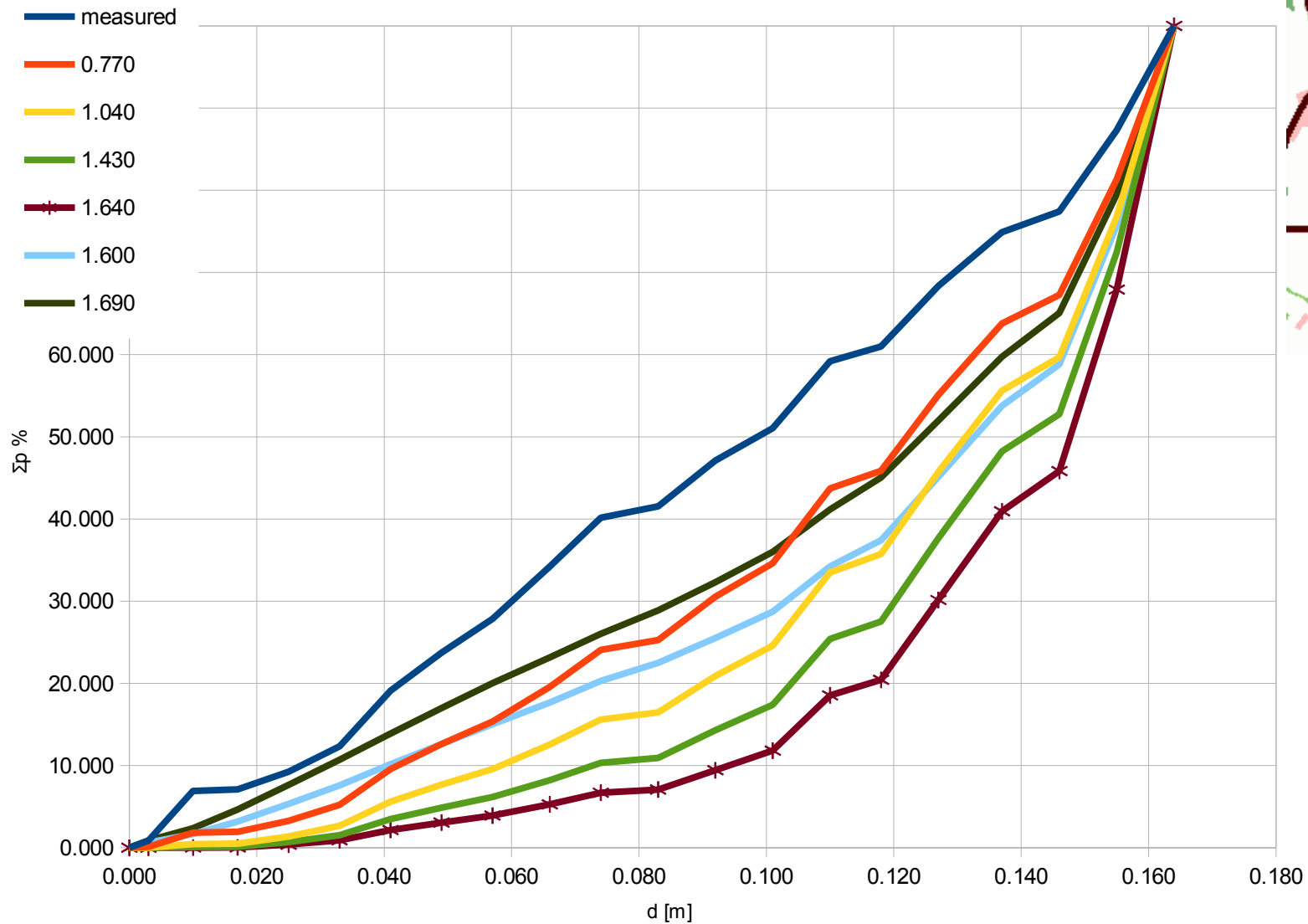
$$\text{for } d_i/d_m > 0.6$$

$$f_m = 0.0123 e^{1.6 SF}$$

Bartnik [1997]

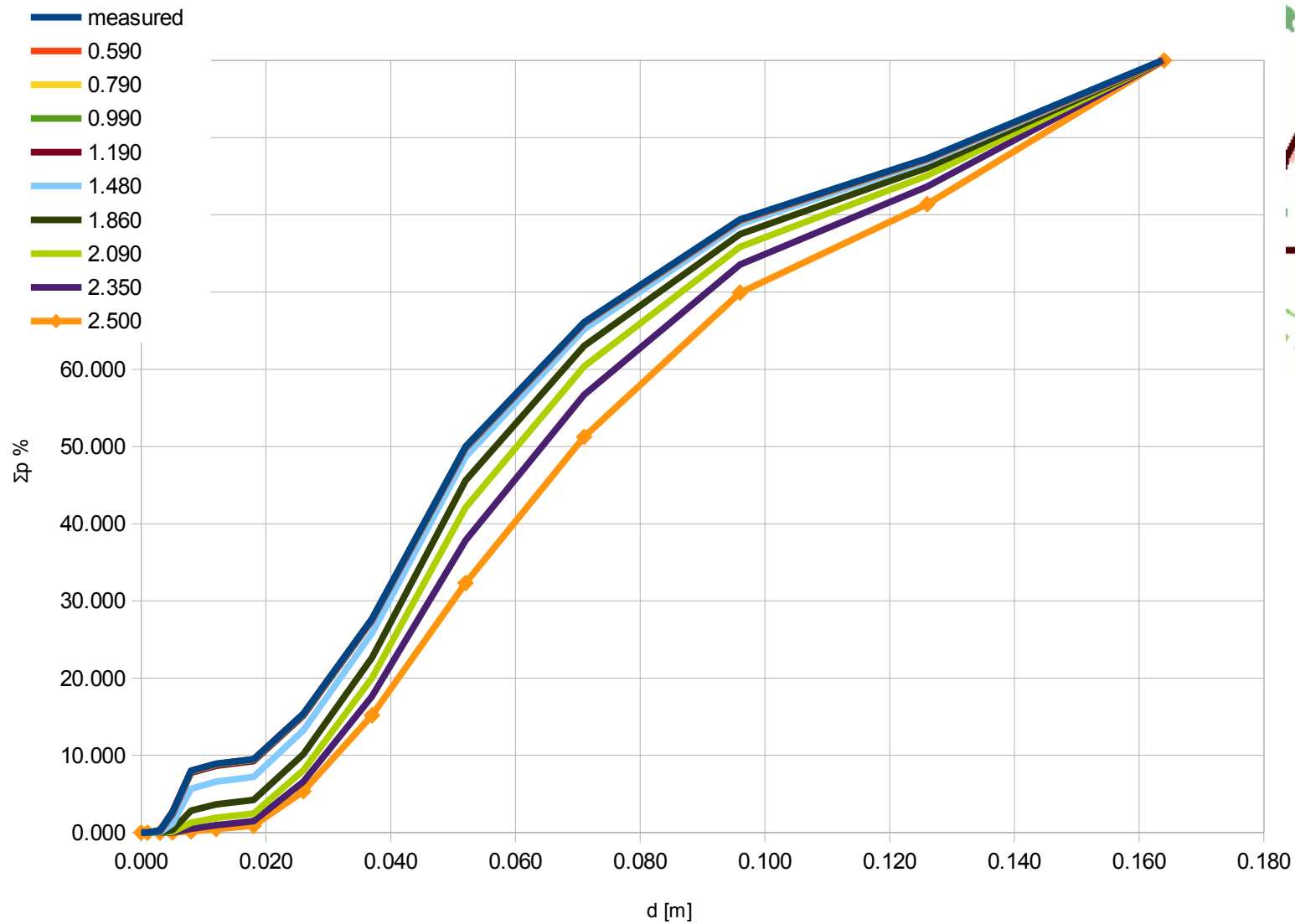
[Bartnik, 1992]

5. Results



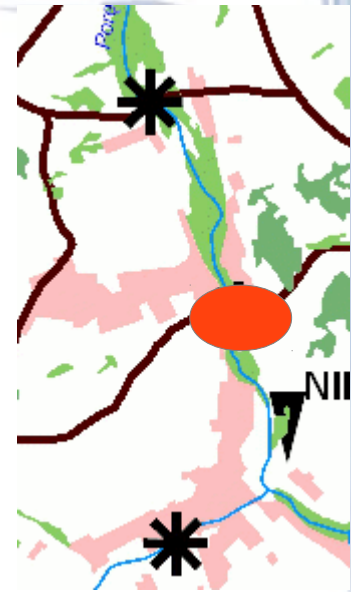
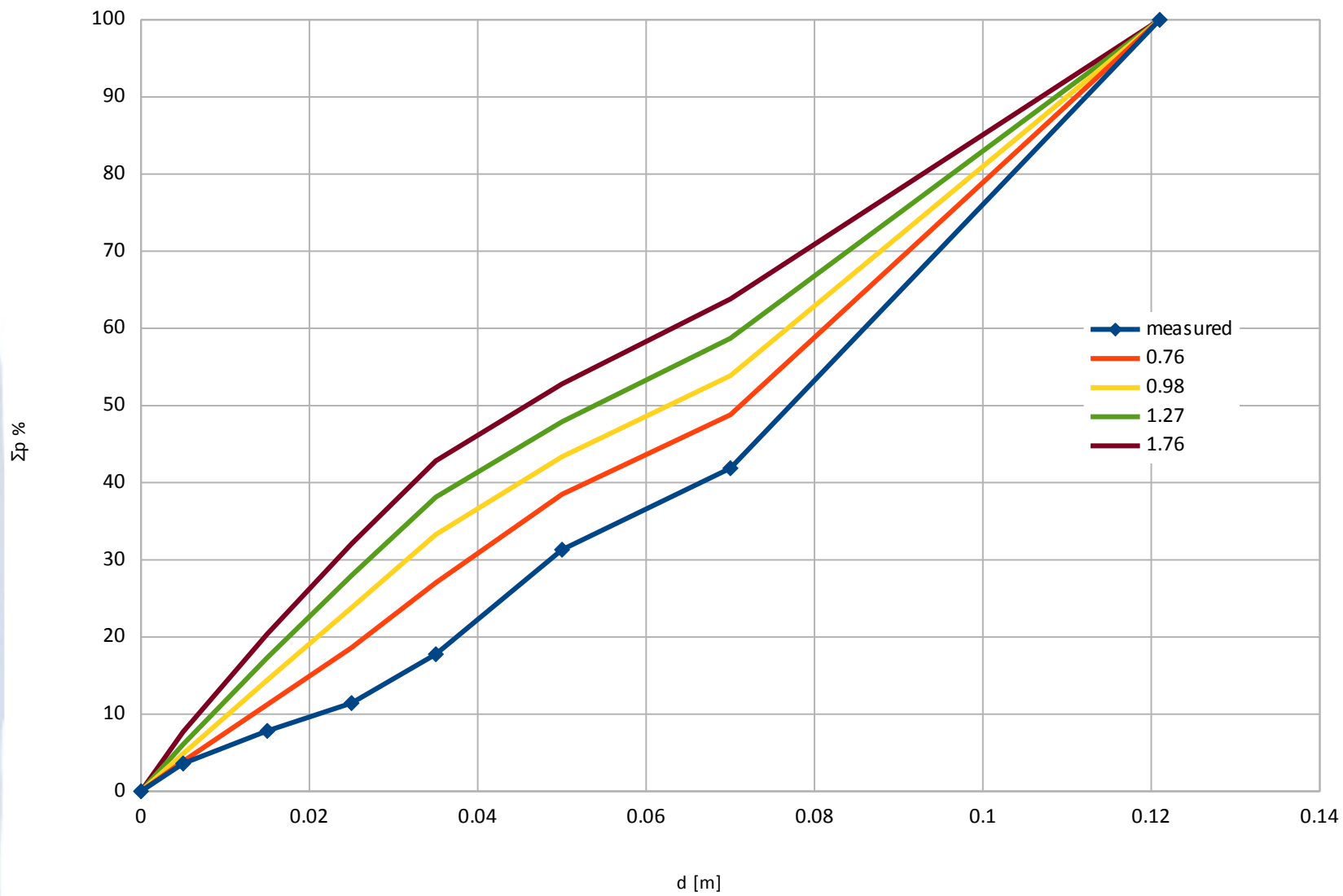
reach I, cross-section 2 – armour coat up to $Q_{5\%}$

5. Results



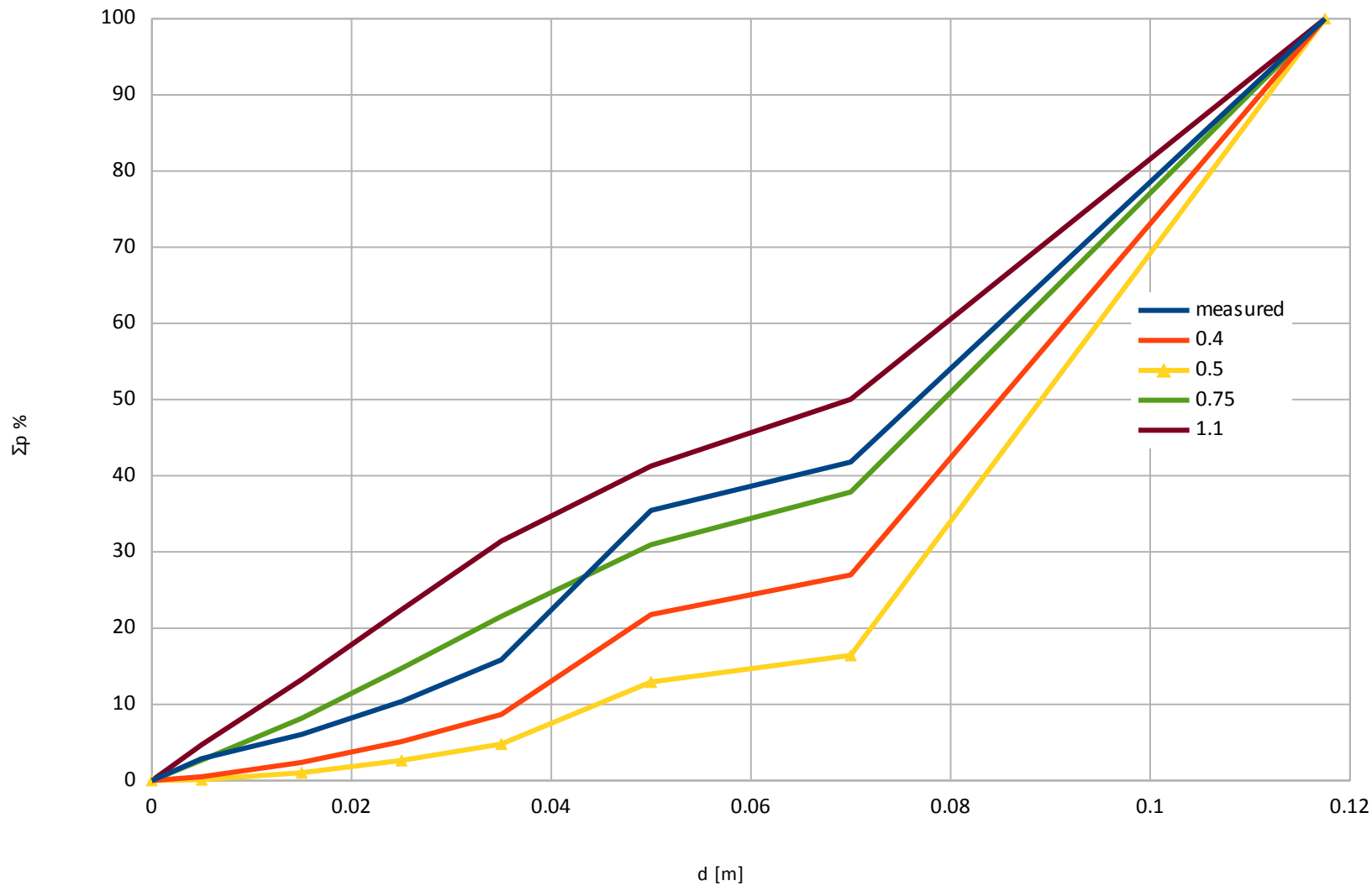
reach I, cross-section 3 – armoring processes coat up to $Q_{1\%}$ and higher

5. Results



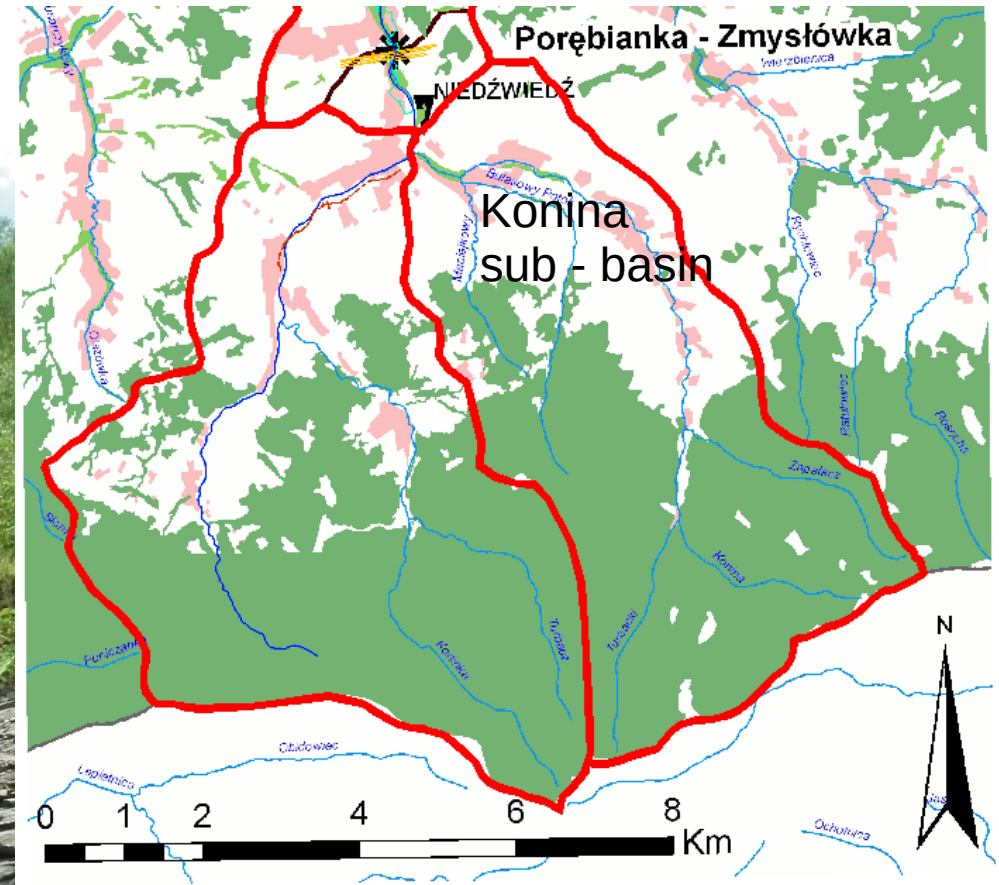
reach II – bed not stable – high transportation reach

5. Results



reach III – armour coat up to $Q_{35\%}$ (dominant discharge often meets $Q_{75\%}$)

6. Results interpretation



bedload alimentation weak due to regulation structures and increased afforestation

6. Results interpretation

fluvial processes are currently poorly balanced

reach I, cross-section II – low transport potentials

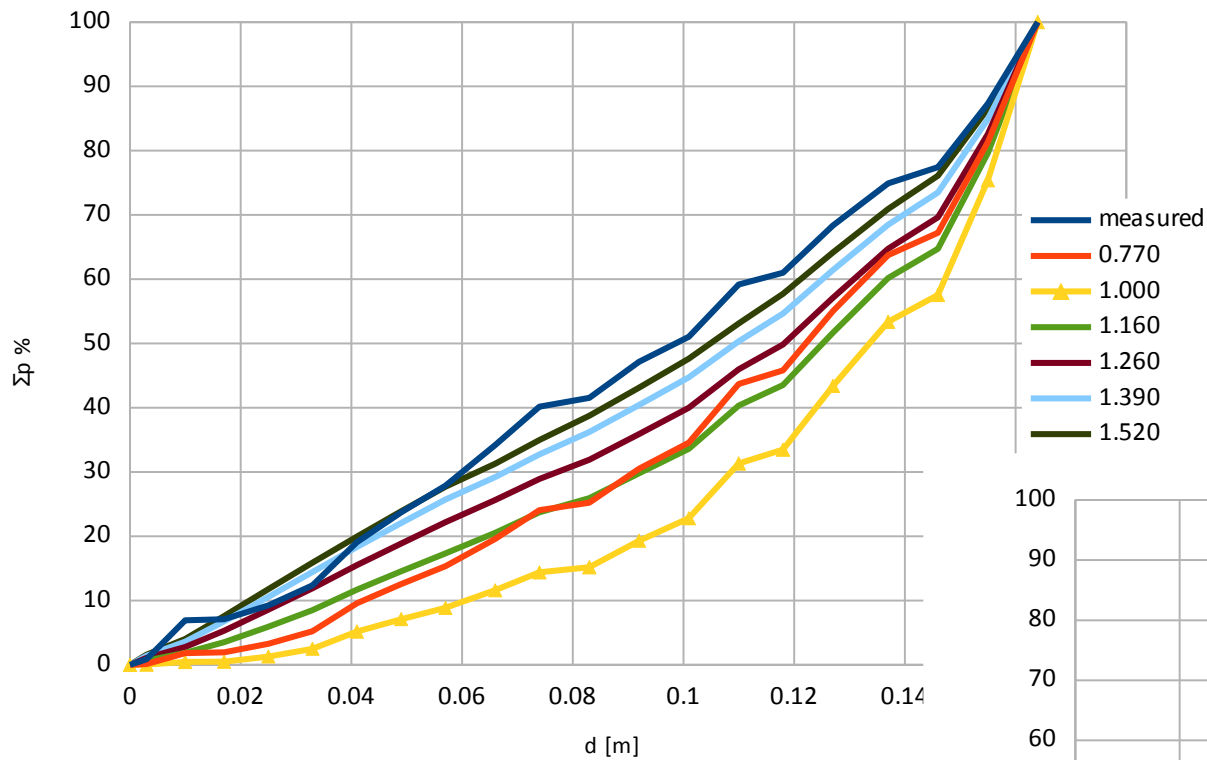
reach I, cross-section III – no transport potentials

Konina stream – low bedload supply

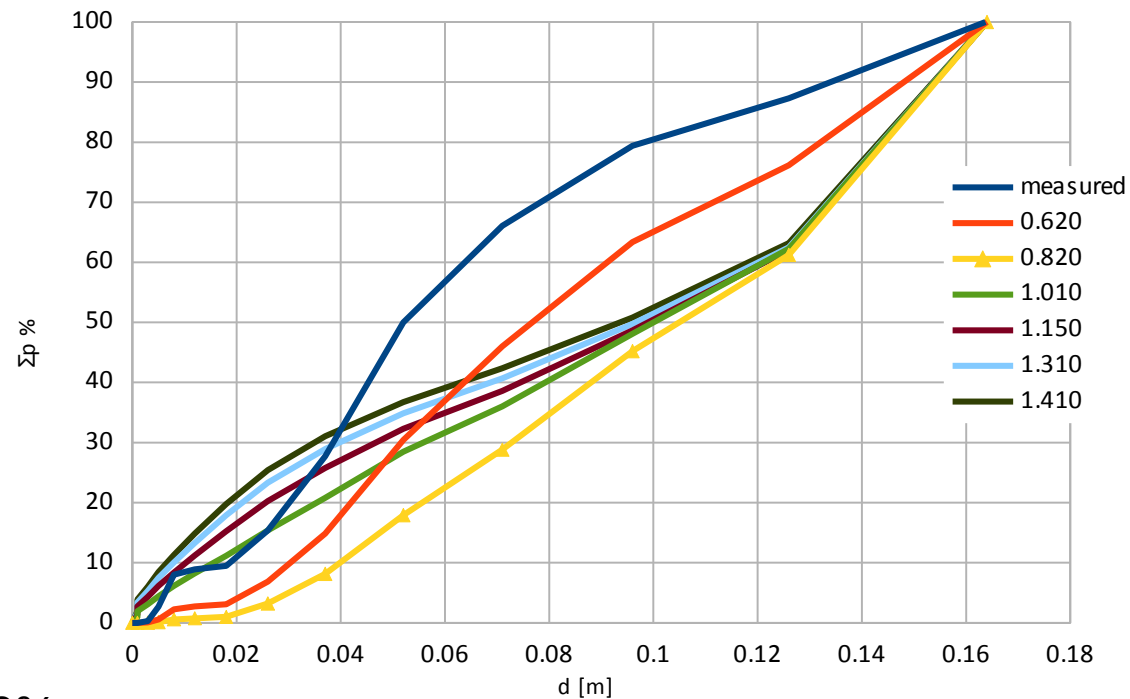
reach II – high transport potentials -> high bed erosion

reach III – average transport potentials

7. Additional solutions



after removing regulation
structures
in the highest reach (I)



Q25%

bedload
alimentation potentials
dramatically increased

less than Q50%
!!!

8. Conclusions

1. Regulations, especially done in the highest reach, destroy river continuity
2. At present high erosion processes appear in transportation reach II from the end of reach I to reach III, especially below outlet of the Konina stream
3. Within reach III effects of decreased bedload alimentation are reduced by ramps
3. After removing of the small steps from reach I much more intensive fluvial processes would appear
 - this could improve the bed condition in reach II but also
 - could also start high movement of bedload and bed erosion within the lower part of reach I



Thank You