



Mathematical modelling of sand-gravel bed evolution in one dimension

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Presentation outline

- 1. Introduction
- 2. The purpose of the research
- 3. Mathematical formulation
- 4. The model verification
- 5. Conclusion



Bed-load sediment mostly forms local-scale bed shape



Introduction

Bed-load transport models



[Bagnold, 1966]

[Shamov, 1959]

[van Rijn, 1986]

[Wu, Wang and Jia, 2000]



The purpose of the research

To model accurately **local-scale** bed evolution, e.g.:

- degradation behind the dam;
- reservoir siltation;
- dunes development.

Water Water Bed dune

Analytical bed-load transport model

E)

Mathematical formulation

The stationary hydrodynamic equations in shallow water approach [Grishanin, K.V., 1974]

(1)
$$\frac{\partial}{\partial x}\left(\frac{U^2}{2g}+\eta\right)+\frac{\tau}{gH\rho_w}=0, \qquad Q=UH,$$

The Chezy-Manning formula for bed shear stress [Grishanin, K.V., 1974] (2) $\tau = \rho_w g \frac{U^2}{C^2}$, $C = \frac{H^{1/6}}{n_c}$, $n_s = \frac{H^{2/3} \sqrt{J}}{II}$.



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Where Q - the water rate, τ - the bed shear stress, n_s - the Manning roughness coefficient.

Mathematical formulation

The Exner equation [Exner, F.M., 1925]

(3)
$$(1-\varepsilon)\rho_s\frac{\partial\zeta}{\partial t} + \frac{\partial G}{\partial x} = -\alpha\frac{W}{H}(S_*-S),$$

The suspended transport equations [Karaushev, A.V., 1977]

(4)
$$\frac{\partial S}{\partial t} + \frac{\partial SU}{\partial x} = \alpha \frac{W}{H} (S_* - S), \quad S_* = \begin{cases} \mathcal{B} \frac{U^3}{WH}, & W < u_*, \\ 0, & W \ge u_*; \end{cases} \quad u_* = \sqrt{\frac{\tau}{\rho_W}}, \end{cases}$$

Phenomenological parameters

Where ρ_s - the water density, ε - the bed material porosity, W - the particle fall velocity, $\alpha \in (0; 1)$ - the adaptation coefficient, $\beta = 0.2$, S_* - the suspended transport capacity

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Mathematical formulation

The analytical bed-load rate formula [Petrov, P.G., 1991]

(5)
$$G = G_0 \tau^{3/2} \left[(1-\chi) - \left(1 - \frac{\chi}{2}\right) \frac{1}{\tan \varphi \cos \gamma} \frac{\partial \zeta}{\partial x} \right],$$

Granulometrical and physical-mechanical parameters of the bed material

Local bed slope

Bed-shear stress

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(6)
$$G_{0} = \frac{4}{3\kappa\sqrt{\rho_{w}}(\rho_{s} - \rho_{w})g} \tan(\rho_{s} - \rho_{w})g} \tan(\rho_{w})g} \tan(\rho_{w})g}$$

Where τ_* - the critical bed-shear stress, φ - the internal friction angle of particles, c_x - the frontal particle drag coefficient, κ - the Karmans constant.



The model verification

In order to estimate the ability of the model to describe localscale bed evolution the next problems are solved:

- local bed degradation behind the dam,
- local bed aggregation,
- trench evolution.

Bed degradation











Bed aggregation













Conclusion

- The proposed model included the analytical bed-load rate formula is verified on three practice problems.
- The proposed model is adequately describes local-bed evolution.
- It has almost the same accuracy as models [Sanchez, A., Wu, W., 2011; Singh, V., 2002; van Rijn, L. C. 1986; Wu, W, 2001; Wu, W., Vieira, D.A., 2002].

<u>Next research</u>

 By using the analytical bed-load rate formula with 2D hydrodynamic and 2D suspended sediment models to research the bed microforms evolution and estimate their influence on the sediment rate and hydraulic resistance.



Thank you!



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